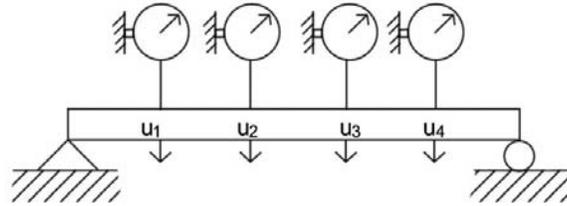


Lecture 15 - Solution of Dynamic Equilibrium Equations

In the last lecture, we described a physical setup that demonstrates the technique of Gauss elimination. We used clamps on each DOF and removed one clamp for one step of Gauss elimination.

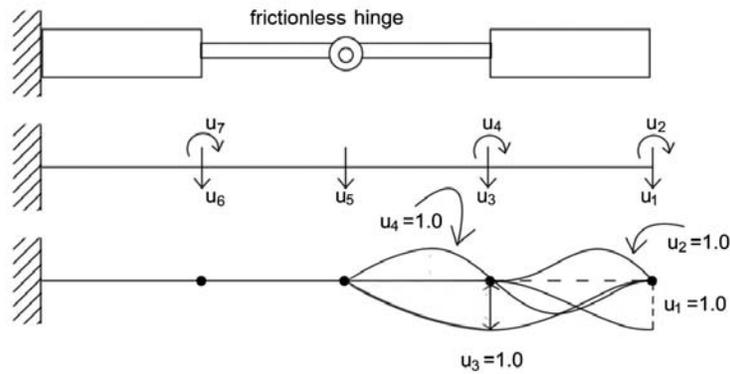


$$\begin{bmatrix} \otimes & \times & \times & \times \\ \times & \otimes & \times & \times \\ \times & \times & \otimes & \times \\ \times & \times & \times & \otimes \end{bmatrix} \begin{bmatrix} u_1 \\ u_2 \\ u_3 \\ u_4 \end{bmatrix}$$

\otimes should be positive, and should remain positive.

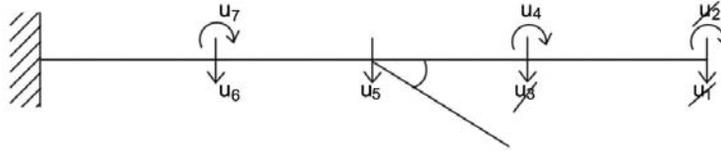
Our rule: Remove clamps one at a time, in the order we would perform Gauss elimination. If there is “a” clamp “seeing” no more stiffness after having removed some clamp(s), the structure is unstable.

Example



$$K = \begin{bmatrix} \times & & & & & & \\ & \times & & & & & \\ & & \times & & & & \\ & & & \times & & & \\ & & & & \times & & \\ & & & & & \times & \\ & & & & & & \times \end{bmatrix}$$

All diagonal terms are positive. However, there will be a zero diagonal entry after Gauss elimination has been performed for the 3rd DOF.



after 3 Gauss elimination of u_1, u_2 and u_3, u_4 sees no stiffness

Solution of dynamic equilibrium equations

Consider a system with n DOFs:

$$M\ddot{U} + C\dot{U} + \underbrace{KU}_{F_I} = R(t) \quad (1)$$

with initial conditions

$$U|_{t=0} = {}^0U \quad ; \quad \dot{U}|_{t=0} = {}^0\dot{U}$$

The term $C\dot{U}$ will be discussed later. Our methods for solving (1) are:

- Mode superposition: We first transform the equation and then integrate.
- Direct integration: We integrate the equation directly!

First, let's transform Eq. (1). Assume we use

$$U(t) = \underset{n \times n}{P} \underset{n \times 1}{X}(t) \quad (2)$$

The function P is independent of time. Substitute this into Eq.(1) to obtain

$$P^T M P \ddot{X} + P^T C P \dot{X} + P^T K P X = P^T R \quad (A)$$

The best P matrix would diagonalize the matrix, thereby decoupling the equations. To obtain a "wonderful" P , consider

$$M\ddot{U} + KU = \mathbf{0} \quad (\text{free vibration})$$

$$U = \phi \sin \omega (t - t_0)$$

Then,

$$-\omega^2 M \phi \sin \omega (t - t_0) + K \phi \sin \omega (t - t_0) = \mathbf{0} \quad (a)$$

For (a) to hold,

$$K \phi = \omega^2 M \phi$$

$$(K - \omega^2 M) \phi = \mathbf{0}$$

Let $\omega^2 = \lambda$. We have a generalized eigenvalue problem. We must have $\det(K - \lambda M) = 0$, and we find the solution for λ from the roots of the characteristic polynomial

$$p(\lambda) = a_0 + a_1 \lambda + a_2 \lambda^2 + \dots + a_n \lambda^n$$

Find the eigenvalues $\lambda_1, \lambda_2, \dots, \lambda_n$ from $p(\lambda) = 0$ and then the eigenvectors ϕ_1, \dots, ϕ_n from

$$(K - \lambda_i M) \phi_i = \mathbf{0}$$

Then, normalize ϕ_i so that it satisfies $\phi_i^T M \phi_i = 1$. We now have (see Chapters 2, 10)

$$0 \leq \underbrace{\omega_1^2}_{\text{for } \phi_1} \leq \underbrace{\omega_2^2}_{\text{for } \phi_2} \leq \dots \leq \underbrace{\omega_n^2}_{\text{for } \phi_n}$$

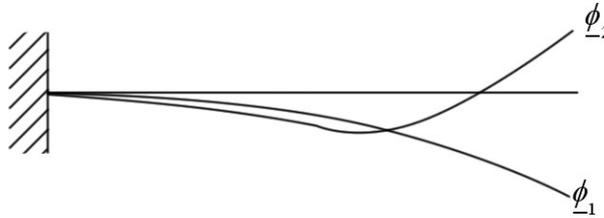
Each ϕ_i represents a mode shape, and we have

$$\phi_i^T \mathbf{M} \phi_j = \delta_{ij}$$

where δ_{ij} is the Kronecker delta, so we call ϕ_i \mathbf{M} -orthogonal (or \mathbf{M} -orthonormal, because $\phi_i^T \mathbf{M} \phi_i = 1$). In turn, this yields

$$\phi_i^T \mathbf{K} \phi_j = \omega_i^2 \delta_{ij}$$

Physically,



Consider ϕ_1 :

$$\phi_1^T \mathbf{M} \phi_1 = 1$$

$$\phi_1^T \mathbf{K} \phi_1 = \omega_1^2$$

The strain energy in the beam is $\frac{1}{2} \phi_1^T \mathbf{K} \phi_1 = \frac{1}{2} \omega_1^2$. By orthonormality, also,

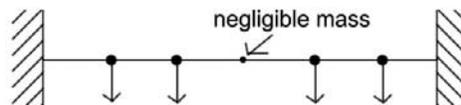
$$\phi_2^T \mathbf{M} \phi_1 = 0$$

$$\phi_2^T \mathbf{M} \phi_2 = 1$$

and

$$\phi_2^T \mathbf{K} \phi_2 = \omega_2^2$$

Consider this simple case, for which we must solve $\mathbf{K} \phi = \omega^2 \mathbf{M} \phi$:



$$\mathbf{M} = \begin{bmatrix} \times & & & & \\ & \times & & & \\ & & 0 & & \\ & & & \times & \\ & & & & \times \end{bmatrix}$$

Then

$$\mathbf{M} \phi = \frac{1}{\omega^2} \mathbf{K} \phi = \kappa \mathbf{K} \phi$$

A non-trivial solution is $\kappa = 0$, $\phi = \begin{bmatrix} 0 \\ 0 \\ 1 \\ 0 \\ 0 \end{bmatrix} \rightarrow \omega^2 = \infty$.

Note: $\omega_1^2 = 0$ for rigid body motion. (No strain energy!)

Now let's use $\mathbf{P} = [\phi_1 \ \dots \ \phi_n]$. Then, (A) becomes

$$\ddot{\mathbf{X}} + \mathbf{P}^T \mathbf{C} \mathbf{P} \dot{\mathbf{X}} + \begin{bmatrix} \omega_1^2 & & \text{zeros} \\ & \ddots & \\ \text{zeros} & & \omega_n^2 \end{bmatrix} \mathbf{X} = \mathbf{P}^T \mathbf{R}$$

For now, let's assume no damping. (If $\mathbf{C} = 0$, there is no damping and the equations are decoupled.) Then, we have

$$\ddot{\mathbf{X}} + \mathbf{\Omega}^2 \mathbf{X} = \mathbf{\Phi}_{n \times n}^T \mathbf{R}$$

$$\mathbf{\Phi} = [\phi_1 \ \phi_2 \ \dots \ \phi_n] \ ; \ \mathbf{\Omega}^2 = \begin{bmatrix} \omega_1^2 & & \text{zeros} \\ & \ddots & \\ \text{zeros} & & \omega_n^2 \end{bmatrix} \ ; \ \mathbf{X} = \begin{bmatrix} x_1 \\ \vdots \\ x_n \end{bmatrix}$$

So, we have

$$\ddot{x}_i + \omega_i^2 x_i = \phi_i^T \mathbf{R} \quad (i = 1, \dots, n)$$

As always, we need the initial conditions ${}^0x_i, {}^0\dot{x}_i$ to solve.

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2.092 / 2.093 Finite Element Analysis of Solids and Fluids I
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