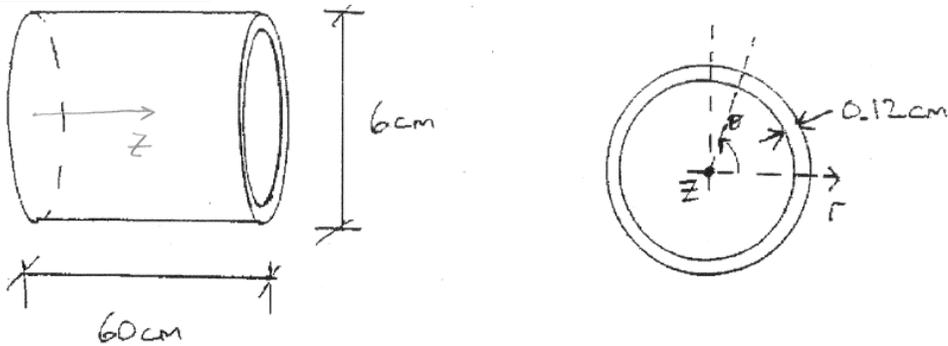


Lecture 2

The Concept of Strain

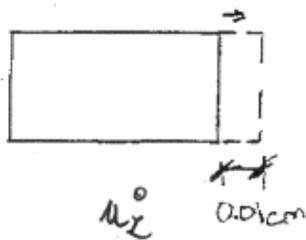
Problem 2-1: A thin-walled steel pipe of length 60 cm, diameter 6 cm, and wall thickness 0.12 cm is stretched 0.01 cm axially, expanded 0.001 cm in diameter, and twisted through 1° . Determine the strain components of the pipe. Note that the shell with a diameter to thickness ratio of 50 is predominately in the membrane state. For plane stress there are only 3 components of the strain tensor.

Problem 2-1 Solution:

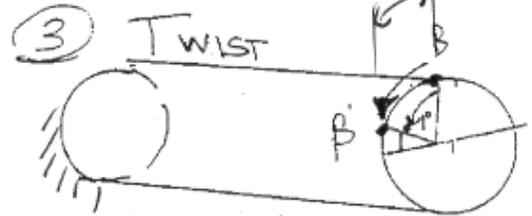
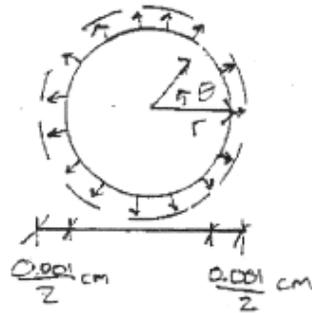


DEFORMED GEOMETRY =

① AXIAL



② RADIAL



Evaluate deformation components at the end cross-section

$$u_r^0 = 0.01 / 2 \text{ cm} = 0.0005 \text{ cm}$$

$$u_\theta^0 = R \cdot \Delta\theta = 3 \cdot (1^\circ \cdot \frac{\pi}{180^\circ}) \text{ cm} = 0.0523 \text{ cm}$$

$$u_z^0 = 0.01 \text{ cm}$$

Evaluate deformation components across the whole pipe

u_r is constant

$$u_r = u_r^0 = 0.0005 \text{ cm}$$

u_θ is linearly proportional to θ and r , independent of z

$$u_\theta = u_\theta(z, r) = u_\theta^0 \frac{z}{L} \frac{r}{R}$$

u_z is linearly proportional to z , independent of r and θ

$$u_z = u_z(z) = u_z^0 \frac{z}{L}$$

Substitute the above equations of u_r , u_θ and u_z into the expression of six components of the strain tensor in cylindrical coordinate system, we have

$$\varepsilon_{rr} = \frac{\partial u_r}{\partial r} = \frac{\partial u_r^\circ}{\partial r} = 0$$

$$\varepsilon_{\theta\theta} = \frac{u_r}{r} + \frac{1}{r} \frac{\partial u_\theta}{\partial \theta} = \frac{u_r^\circ}{r} + \frac{1}{r} \cdot 0 = \frac{0.0005}{6/2} = 1.67 \times 10^{-4}$$

$$\varepsilon_{zz} = \frac{\partial u_z}{\partial z} = \frac{u_z^\circ}{L} = \frac{0.01}{60} = 1.67 \times 10^{-4}$$

$$\varepsilon_{r\theta} = \frac{1}{2} \left(\frac{\partial u_r}{\partial \theta} \frac{1}{r} + \frac{\partial u_\theta}{\partial r} - \frac{u_\theta}{r} \right) = \frac{1}{2} \left(0 + \frac{u_\theta^\circ z}{LR} - \frac{u_\theta^\circ z}{LR} \right) = 0$$

$$\varepsilon_{\theta z} = \frac{1}{2} \left(\frac{\partial u_z}{r \partial \theta} + \frac{\partial u_\theta}{\partial z} \right) = \frac{1}{2} \left(0 + \frac{u_\theta^\circ r}{LR} \right) \approx \frac{1}{2} \frac{u_\theta^\circ R}{LR} = \frac{1}{2} \frac{0.0523}{60} = 4.36 \times 10^{-4}$$

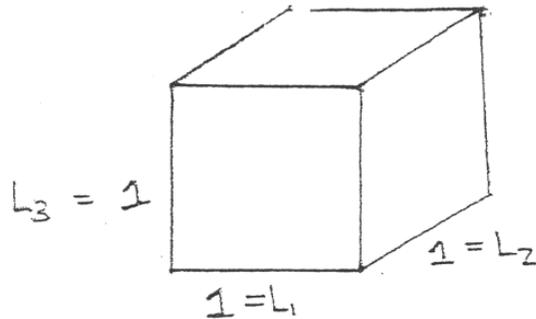
$$\varepsilon_{zr} = \frac{1}{2} \left(\frac{\partial u_r}{\partial z} + \frac{\partial u_z}{\partial r} \right) = \frac{1}{2} \left(\frac{\partial u_r^\circ}{\partial z} + \frac{\partial u_z(z)}{\partial r} \right) = 0$$

Summary

$\varepsilon_{rr} = \varepsilon_{r\theta} = \varepsilon_{zr} = 0$ $\varepsilon_{zz} = 1.67 \times 10^{-4}$ $\varepsilon_{\theta\theta} = 1.67 \times 10^{-4}$ $\varepsilon_{z\theta} = 4.36 \times 10^{-4}$

Problem 2-2: Derive an expression for the change in volume of a unit volume element subjected to an arbitrary small strain tensor.

Problem 2-2 Solution:



A unit volume has sides of unit length

$$\text{Unit Volume} = 1 \times 1 \times 1 = 1[\text{length}]^3$$

Strain is defined by

$$\varepsilon = \frac{\Delta}{L}$$

If each side is deformed by δ , then the new length of each side is $1 + \delta$

$$\varepsilon = \frac{\delta}{L}$$

$$\delta = L\varepsilon = 1 \times \varepsilon$$

Volume after deformation is:

$$V = (1 + \varepsilon_1)(1 + \varepsilon_2)(1 + \varepsilon_3)$$

$$= 1 + \varepsilon_1 + \varepsilon_2 + \varepsilon_3 + \cancel{\varepsilon_1\varepsilon_2} + \cancel{\varepsilon_2\varepsilon_3} + \cancel{\varepsilon_3\varepsilon_1} + \cancel{\varepsilon_1\varepsilon_2\varepsilon_3}$$

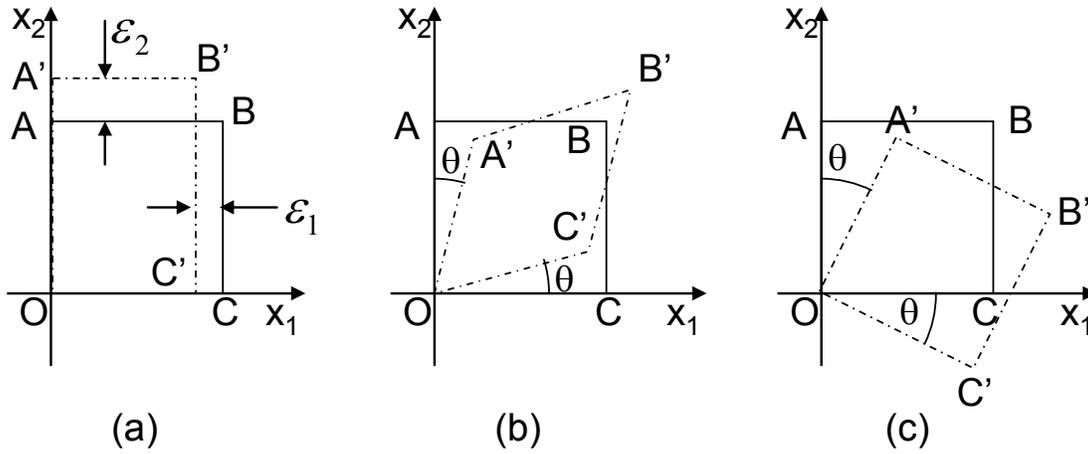
Because of **small** strain, we cancel all higher order terms

Volumetric Strain = change in volume per unit volume, the original volume of a unit volume element is 1

$$\varepsilon_v = \frac{V - V_o}{V_o} = \frac{(1 + \varepsilon_1 + \varepsilon_2 + \varepsilon_3) - 1}{1} = \varepsilon_1 + \varepsilon_2 + \varepsilon_3$$

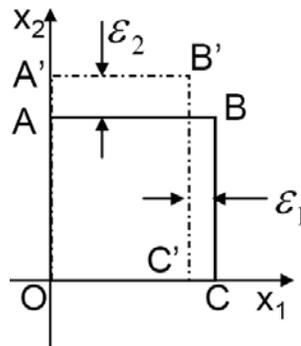
$$\boxed{\varepsilon_v = \varepsilon_1 + \varepsilon_2 + \varepsilon_3}$$

Problem 2-3: A unit square $OABC$ is distorted to $OA'B'C'$ in three ways, as shown in the figure below. In each case write down the displacement field (u_1, u_2) of every point in the square as a function of the location of that point (x_1, x_2) and the strain components ϵ_{ij} .



Problem 2-3 Solution:

(a)



Only **Axial deformation** in this case

$$u_1 = -\epsilon_1 x_1$$

$$u_2 = \epsilon_2 x_2$$

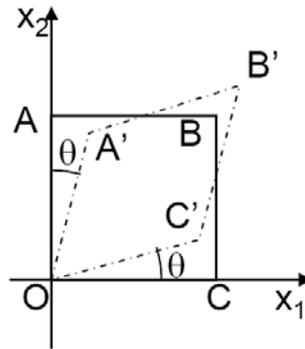
The strain components are

$$\varepsilon_{11} = \frac{\partial u_1}{\partial x_1} = -\varepsilon_1$$

$$\varepsilon_{22} = \frac{\partial u_2}{\partial x_2} = \varepsilon_2$$

$$\varepsilon_{12} = \varepsilon_{21} = \frac{1}{2} \left(\frac{\partial u_1}{\partial x_2} + \frac{\partial u_2}{\partial x_1} \right) = 0$$

(b)



Only **pure shear** in this case

$$\tan \theta = \frac{u_1}{x_2}$$

$$\tan \theta = \frac{u_2}{x_1}$$

Deformation

$$u_1 = \theta x_2$$

$$u_2 = \theta x_1$$

Because of small angle \$\theta\$, \$\tan \theta \approx \theta\$

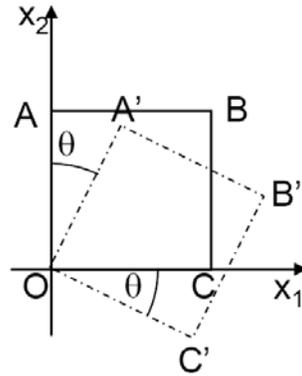
The strain components are

$$\varepsilon_{11} = \frac{\partial u_1}{\partial x_1} = 0$$

$$\varepsilon_{22} = \frac{\partial u_2}{\partial x_2} = 0$$

$$\varepsilon_{12} = \varepsilon_{21} = \frac{1}{2} \left(\frac{\partial u_1}{\partial x_2} + \frac{\partial u_2}{\partial x_1} \right) = \theta$$

(c)



Only **rigid body rotation** in this case, there should be 0 strain everywhere

Deformation

$$u_1 = \theta x_2$$

$$u_2 = -\theta x_1$$

The strain components are

$$\varepsilon_{11} = \frac{\partial u_1}{\partial x_1} = 0$$

$$\varepsilon_{22} = \frac{\partial u_2}{\partial x_2} = 0$$

$$\varepsilon_{12} = \varepsilon_{21} = \frac{1}{2} \left(\frac{\partial u_1}{\partial x_2} + \frac{\partial u_2}{\partial x_1} \right) = \frac{1}{2} (\theta - \theta) = 0$$

Problem 2-4: The strain (plane strain) in a given point of a body is described by the 2x2 matrix below. Find components of the strain tensor $\varepsilon_{x'y'}$ in a new coordinate system rotated by the angle θ . Consider four cases $\theta = 45^\circ, -45^\circ, 60^\circ$ and -60° .

$$\varepsilon_{xy} = \begin{bmatrix} 0 & 0.05 \\ 0.05 & 0 \end{bmatrix}$$

Problem 2-4 Solution:

Transformation equations for plane strain, from old coordinate system xy to new coordinate system $x'y'$

$$\varepsilon_{x'x'} = \frac{\varepsilon_{xx} + \varepsilon_{yy}}{2} + \frac{\varepsilon_{xx} - \varepsilon_{yy}}{2} \cos(2\theta) + \varepsilon_{xy} \sin(2\theta)$$

$$\varepsilon_{y'y'} = \frac{\varepsilon_{xx} + \varepsilon_{yy}}{2} - \frac{\varepsilon_{xx} - \varepsilon_{yy}}{2} \cos(2\theta) - \varepsilon_{xy} \sin(2\theta)$$

$$\varepsilon_{x'y'} = -\frac{\varepsilon_{xx} - \varepsilon_{yy}}{2} \sin(2\theta) + \varepsilon_{xy} \cos(2\theta)$$

Given strain components:

$$\varepsilon_x = \varepsilon_y = 0$$

$$\varepsilon_{xy} = 0.05$$

Simplify the transformation equations:

$$\varepsilon_{x'x'} = \varepsilon_{xy} \sin(2\theta)$$

$$\varepsilon_{y'y'} = -\varepsilon_{xy} \sin(2\theta)$$

$$\varepsilon_{x'y'} = \varepsilon_{xy} \cos(2\theta)$$

Substitute the strain components into the simplified transformation equations:

$$\theta = 45^\circ$$

$$\varepsilon_{x'y'} = \begin{bmatrix} 0.05 & 0 \\ 0 & -0.05 \end{bmatrix}$$

$$\theta = -45^\circ$$

$$\varepsilon_{x'y'} = -\begin{bmatrix} 0.05 & 0 \\ 0 & -0.05 \end{bmatrix}$$

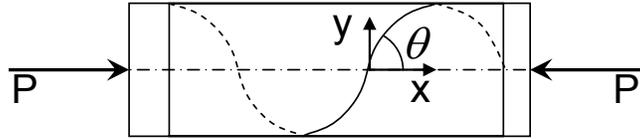
$$\theta = 60^\circ$$

$$\varepsilon_{x'y'} = \begin{bmatrix} 0.04 & -0.025 \\ -0.025 & -0.04 \end{bmatrix}$$

$$\theta = -60^\circ$$

$$\varepsilon_{x'y'} = \begin{bmatrix} -0.04 & -0.025 \\ -0.025 & 0.04 \end{bmatrix}$$

Problem 2-5: A cylindrical pipe of 160-mm outside diameter and 10-mm thickness, spirally welded at an angle of $\theta=40^\circ$ with the axial (x) direction, is subjected to an axial compressive load of $P=150$ kN through the rigid end plates (see below). Determine the normal force $\sigma_{x'}$ and shearing stresses τ_{xy} acting simultaneously in the plane of the weld.



Problem 2-5 Solution:

Calculate stress in given coordinate system

Compressive force in x-direction

$$\sigma_{xx} = -\frac{P}{A} = -\frac{150\text{kN}}{\pi(80^2 - 70^2)(10^{-3})^2 \text{cm}^2}$$

The stress components are

$$\begin{aligned}\sigma_{xx} &= -32\text{MPa} \\ \sigma_{yy} &= 0 \\ \sigma_{xy} &= 0\end{aligned}$$

Transformation equations for plane strain

$$\begin{aligned}\varepsilon_{x'x'} &= \frac{\varepsilon_{xx} + \varepsilon_{yy}}{2} + \frac{\varepsilon_{xx} - \varepsilon_{yy}}{2} \cos(2\theta) + \varepsilon_{xy} \sin(2\theta) \\ \varepsilon_{y'y'} &= \frac{\varepsilon_{xx} + \varepsilon_{yy}}{2} - \frac{\varepsilon_{xx} - \varepsilon_{yy}}{2} \cos(2\theta) - \varepsilon_{xy} \sin(2\theta) \\ \varepsilon_{xy} &= -\frac{\varepsilon_{xx} - \varepsilon_{yy}}{2} \sin(2\theta) + \varepsilon_{xy} \cos(2\theta)\end{aligned}$$

Substitute the strain components and $\theta = 40^\circ$ into the simplified transformation equations:

$\sigma_{x'x'} = -18.77\text{MPa}$
$\sigma_{y'y'} = -13.22\text{MPa}$
$\sigma_{z'z'} = -15.76\text{MPa}$

Problem 2-6: A displacement field in a body is given by:

$$\begin{aligned}u &= c(x^2 + 10) \\v &= 2cyz \\w &= c(-xy + z^2)\end{aligned}$$

where $c=10^{-4}$. Determine the state of strain on an element positioned at $(0, 2, 1)$.

Problem 2-6 Solution:

Substitute the displacement into the expression of strain components, we have

$$\varepsilon_{xx} = \frac{\partial u}{\partial x} = c \cdot 2x = 10^{-4} \cdot 2 \cdot 0 = 0$$

$$\varepsilon_{yy} = \frac{\partial v}{\partial y} = 2cz = 2 \cdot 10^{-4} \cdot 1 = 2 \times 10^{-4}$$

$$\varepsilon_{zz} = \frac{\partial w}{\partial z} = 2cz = 2 \cdot 10^{-4} \cdot 1 = 2 \times 10^{-4}$$

$$\varepsilon_{xy} = \frac{1}{2} \left(\frac{\partial u}{\partial y} + \frac{\partial v}{\partial x} \right) = \frac{1}{2} (0 + 0) = 0$$

$$\varepsilon_{yz} = \frac{1}{2} \left(\frac{\partial v}{\partial z} + \frac{\partial w}{\partial y} \right) = \frac{1}{2} (2cy - cx) = \frac{1}{2} (2 \cdot 2 - 0) = 2 \times 10^{-4}$$

$$\varepsilon_{xz} = \frac{1}{2} \left(\frac{\partial u}{\partial z} + \frac{\partial w}{\partial x} \right) = \frac{1}{2} (0 - cy) = -1 \times 10^{-4}$$

At $(0, 2, 1)$, the state of strain is

$$\underline{\underline{\varepsilon}} = \begin{bmatrix} 0 & 0 & -1 \times 10^{-4} \\ 0 & 2 \times 10^{-4} & 2 \times 10^{-4} \\ -1 \times 10^{-4} & 2 \times 10^{-4} & 2 \times 10^{-4} \end{bmatrix}$$

Problem 2-7: The distribution of stress in an aluminum machine component is given by:

$$\begin{aligned}\sigma_x &= d(cy + 2z^2), & \tau_{xy} &= 3dz^2 \\ \sigma_y &= d(cx + cz), & \tau_{yz} &= dx^2 \\ \sigma_z &= d(3cx + cy), & \tau_{xz} &= 2dy^2\end{aligned}$$

where $c=1$ mm and $d=1\text{MPa/mm}^2$. Calculate the state of strain of a point positioned at $(1, 2, 4)$ mm. Use $E=70$ GPa and $\nu=0.3$.

Problem 2-7 Solution:

Substitute the given value of c, d, E, ν into stress component expressions, we have

$$\begin{aligned}\sigma_x &= 34\text{MPa} & \tau_{yx} &= 48\text{MPa} \\ \sigma_y &= 5\text{MPa} & \tau_{yz} &= 1\text{MPa} \\ \sigma_z &= 34\text{MPa} & \tau_{xz} &= 8\text{MPa}\end{aligned}$$

Use constitutive equations

$$\begin{aligned}\varepsilon_{xx} &= \frac{1}{E} [\sigma_{xx} - \nu(\sigma_{yy} + \sigma_{zz})] & \gamma_{xy} &= \frac{1}{G} \tau_{xy} \\ \varepsilon_{yy} &= \frac{1}{E} [\sigma_{yy} - \nu(\sigma_{xx} + \sigma_{zz})] & \gamma_{yz} &= \frac{1}{G} \tau_{yz} \\ \varepsilon_{zz} &= \frac{1}{E} [\sigma_{zz} - \nu(\sigma_{yy} + \sigma_{xx})] & \gamma_{xz} &= \frac{1}{G} \tau_{xz}\end{aligned}$$

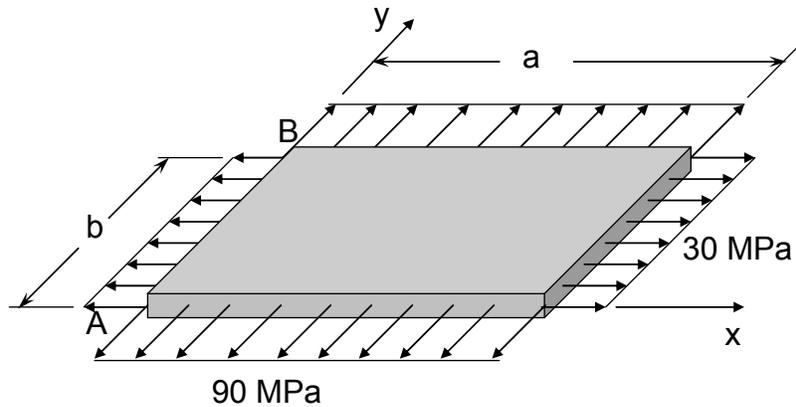
And $G = \frac{E}{2(1+\nu)}$

We have the strain components

$$\begin{aligned}\varepsilon_{xx} &= 4.42 \times 10^{-4} \\ \varepsilon_{yy} &= -9.57 \times 10^{-5} \\ \varepsilon_{zz} &= -9.57 \times 10^{-5}\end{aligned}$$

$$\begin{aligned}\gamma_{xy} &= 8.91 \times 10^{-4} \\ \gamma_{yz} &= 1.86 \times 10^{-5} \\ \gamma_{xz} &= 1.49 \times 10^{-4}\end{aligned}$$

Problem 2-8: An aluminum alloy plate ($E=70$ GPa, $\nu=1/3$) of dimensions $a=300$ mm, $b=400$ mm, and thickness $t=10$ mm is subjected to biaxial stresses as shown below. Calculate the change in (a) the length AB; (b) the volume of the plate.



Problem 2-8 Solution:

The state of stress is given

$$\sigma_{xx} = 30\text{MPa}$$

$$\sigma_{yy} = 90\text{MPa}$$

$$\sigma_{zz} = 0\text{MPa}$$

Use constitutive equations to calculate strains ϵ_{xx} and ϵ_{yy}

$$\epsilon_{xx} = \frac{1}{E} [\sigma_{xx} - \nu(\sigma_{yy} + \sigma_{zz})] = \frac{1}{70 \times 10^9} \left[30 - \frac{1}{3} \times 90 - 0 \right] \times 10^6 = 0$$

$$\epsilon_{yy} = \frac{1}{E} [\sigma_{yy} - \nu(\sigma_{xx} + \sigma_{zz})] = \frac{1}{70 \times 10^9} \left[90 - \frac{1}{3} \times 30 - 0 \right] \times 10^6 = 0.00114$$

$$\epsilon_{zz} = \frac{1}{E} [\sigma_{zz} - \nu(\sigma_{xx} + \sigma_{yy})] = \frac{1}{70 \times 10^9} \left[0 - \frac{1}{3} (30 + 90) \right] = -0.00057$$

Given strain ϵ_{yy} , calculate the change in length in y

$$\epsilon_{yy} = \frac{\Delta_y}{L_o}$$

$$\Delta_y = L_o \epsilon_{yy}$$

$$\Delta_y = 0.46\text{mm}$$

Using solutions in problem 2-2, calculate the change in volume

$$\varepsilon_v = \frac{\Delta V}{V_0}$$

$$\Rightarrow \Delta V = V_0 \varepsilon_v = V_0 (\varepsilon_{xx} + \varepsilon_{yy} + \varepsilon_{zz}) = a \times b \times t \times (\varepsilon_{xx} + \varepsilon_{yy} + \varepsilon_{zz})$$

$$\boxed{\Delta V = 685.72 \text{ mm}^3}$$

Problem 2-9: Consider the general definition of the strain tensor in the 3D continuum. The three Euler-Bernoulli hypotheses of the elementary beam theory state:

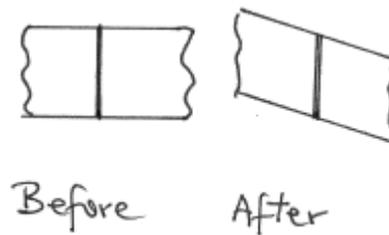
1. Plane remain plane
2. Normal remain normal
3. Transverse deflections are only a function of the length coordinate x .

Proof that under the above assumptions the state in the beam is uni-axial, meaning that the only surviving component of the strain is in the length, x -direction. The proof is sketched in the lecture notes, but we want you to redo the step by step derivation.

Problem 2-9 Solution:

1. Plane remains Plane

This Hypothesis is satisfied when the u -component of the displacement vector is a linear function of z

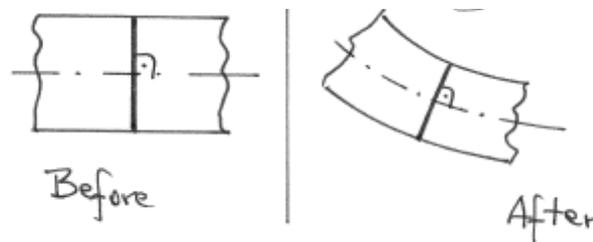


$$u_z = u^0 - \theta z \tag{1}$$

where u^0 is the displacement of the beam axis

2. Normal remains normal

This hypothesis is satisfied when the rotation of the deformed cross-section θ is equal to the local slope of the middle axis



$$\theta = \frac{dw}{dx} \quad (2)$$

Combing (1) and (2), we have

$$u(x, z) = u^o - \frac{dw}{dx} z \quad (3)$$

3. Deflections are only a function of x

$$w = w(x) \quad (4)$$

Also, because of planar deformation

$$v \equiv 0 \quad (5)$$

Evaluate all components of strain tensor

$$\varepsilon_{xx} = \frac{\partial u}{\partial x}$$

$$\text{From equation(5): } v = 0 \Rightarrow \boxed{\varepsilon_{yy} = \frac{\partial v}{\partial y} = 0}$$

$$\text{From equation(4): } w = w(x) \Rightarrow \boxed{\varepsilon_{zz} = \frac{\partial w(x)}{\partial z} = 0}$$

$$\text{From equation(4): } w = w(x) \text{ and equation(5): } v = 0 \Rightarrow \boxed{\varepsilon_{yz} = \frac{1}{2} \left(\frac{\partial v}{\partial z} + \frac{\partial w(x)}{\partial y} \right) = \frac{1}{2} (0 + 0) = 0}$$

$$\text{From equation(3): } u = u^o - \frac{dw}{dx} z \Rightarrow \boxed{\varepsilon_{zx} = \frac{1}{2} \left(\frac{\partial u}{\partial z} + \frac{\partial w}{\partial x} \right) = \frac{1}{2} \left(-\frac{\partial w}{\partial x} + \frac{\partial w}{\partial x} \right) = 0}$$

$$\text{From equation(3): } u = u(x, z) \text{ and equation(5): } v = 0 \Rightarrow \boxed{\varepsilon_{xy} = \frac{1}{2} \left(\frac{\partial u(x, z)}{\partial y} + \frac{\partial v}{\partial x} \right) = \frac{1}{2} (0 + 0) = 0}$$

Only ε_{xx} survives, therefore the state of strain of a classical beam theory is uniaxial

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