
Special Topics in Structures: Residual Stress and Energy Methods

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- * With thanks to Steve Senturia, from whose lecture notes some of these materials are adapted.**

Outline

- > **Effects of residual stresses on structures**
- > **Energy methods**
 - **Elastic energy**
 - **Principle of virtual work: variational methods**
 - **Examples**
- > **Rayleigh-Ritz methods for resonant frequencies and extracting lumped-element masses for structures**

Reminder: Thin Film Stress

- > If a thin film is **adhered to a substrate**, mismatch of thermal expansion coefficient between film and substrate can lead to stresses in the film (and, to a lesser degree, stresses in the substrate)
- > Residual stress can also come from film structure: **intrinsic stress**
- > Stresses set up bending moments that can bend the substrate
- > When we release a residually stressed MEMS structure, interesting effects can ensue

Reminder: Differential equation of beam bending

> Small angle bending:

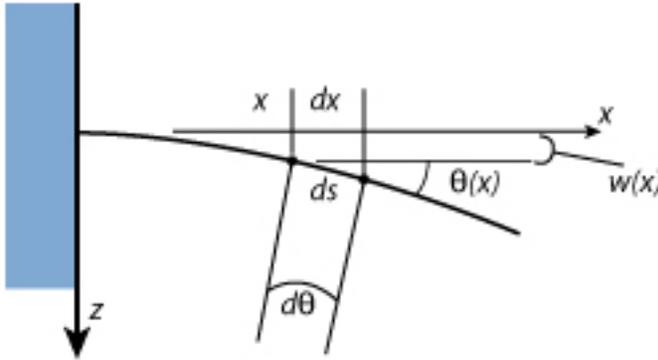


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Adapted from Figure 9.11 in: Senturia, Stephen D. *Microsystem Design*. Boston, MA: Kluwer Academic Publishers, 2001, pp. 214.
ISBN: 9780792372462.

> Beam equation:

- q = distributed load
- w = vertical displacement
- x = axial position along beam

$$\frac{d^4 w}{dx^4} = \frac{q}{EI}$$

Example: Fixed-fixed beam

- > **Fixed-fixed beams are common in MEMS: switches, diffraction gratings, flexures**
- > **Example: Silicon Light Machines Grating Light Valve display deflects a beam in order to diffract light**

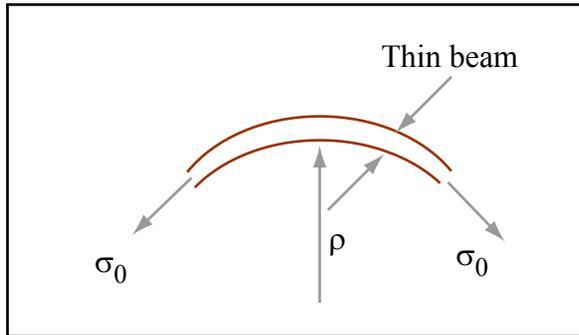
Image removed due to copyright restrictions.

Please see: Figure 1.4 in Senturia, Stephen D. *Microsystem Design*. Boston, MA: Kluwer Academic Publishers, 2001, p. 7. ISBN: 9780792372462.

- > **Residual stress in beams can enhance or reduce response to an applied load, and impact flatness of actuated beam**
- > **Residual stress can be included in the basic beam bending equation by the addition of an extra term**

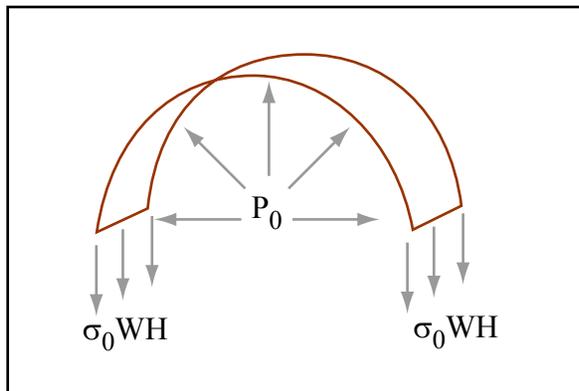
Residual Axial Stress in Beams

- > Residual axial stress in a beam contributes to its bending stiffness
- > Leads to the Euler beam equation



Images by MIT OpenCourseWare.

Adapted from Figure 9.15 in Senturia, Stephen D. *Microsystem Design*. Boston, MA: Kluwer Academic Publishers, 2001, p. 227. ISBN: 9780792372462.



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Adapted from Figure 9.16 in Senturia, Stephen D. *Microsystem Design*. Boston, MA: Kluwer Academic Publishers, 2001, p. 228. ISBN: 9780792372462.

$$2\rho WP_0 = 2\sigma_0 WH$$

$$\Rightarrow P_0 = \frac{\sigma_0 H}{\rho}$$

which is equivalent to a distributed load

$$q_0 = P_0 W = \sigma_0 WH \frac{d^2 w}{dx^2}$$

Insert as added load into beam equation :

$$EI \frac{d^4 w}{dx^4} = q + q_0$$

$$EI \frac{d^4 w}{dx^4} - \sigma_0 WH \frac{d^2 w}{dx^2} = q$$

Example: Effect of tensile stress on stiffness

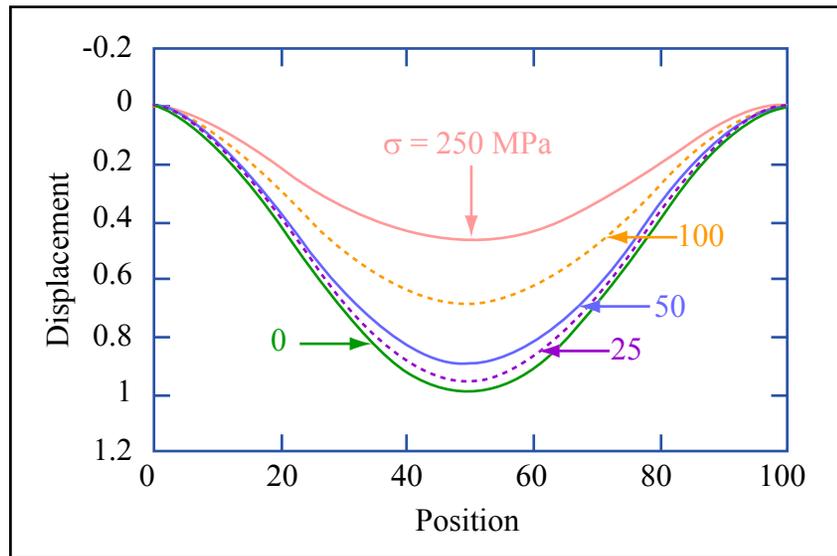


Image by MIT OpenCourseWare.

Adapted from Figure 9.17 in Senturia, Stephen D. *Microsystem Design*.
Boston, MA: Kluwer Academic Publishers, 2001, p. 232. ISBN: 9780792372462.

100 μm long, 2 μm wide, 2 μm high fixed-fixed silicon beam

Stress impacts flatness: leveraged bending

- > Pull-in is modified if the actuating electrodes are away from the point of closest approach

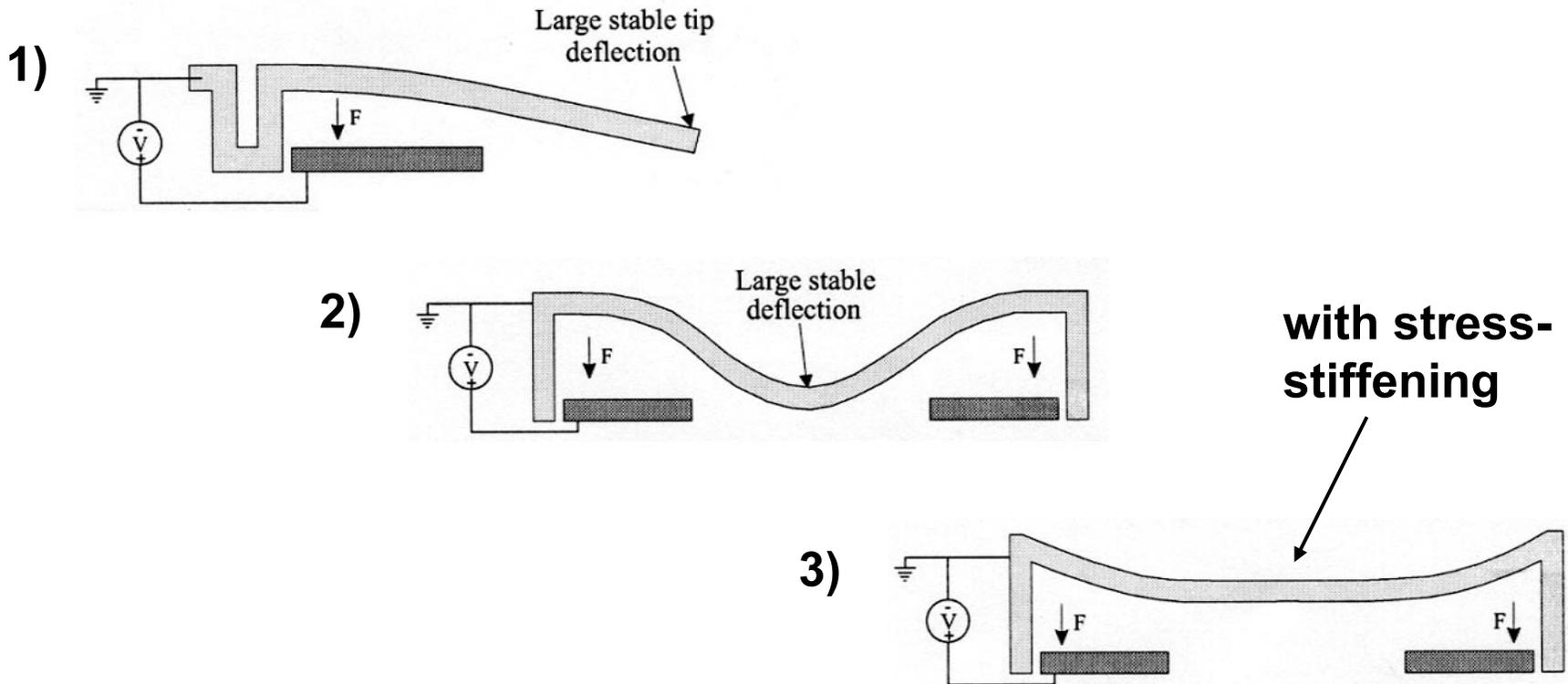


Figure 3 on p. 499 in: Hung, E. S., and S. D. Senturia. "Extending the Travel Range of Analog-tuned Electrostatic Actuators." *Journal of Microelectromechanical Systems* 8, no. 4 (December 1999): 497-505. © 1999 IEEE.

Buckling of Axially Loaded Beams

- > If the compressive stress is too large, a beam will spontaneously bend – this is called *buckling*
- > The basic theory of buckling is in Sec. 9.6.3
- > The Euler buckling criterion:

$$\sigma_{Euler} = -\frac{\pi^2}{3} \frac{EH^2}{L^2}$$

Plates with in-plane stress and membranes

- > As with the Euler beam equation, in plane stress can be included

$$D \left(\frac{\partial^4 w}{\partial x^4} + 2 \frac{\partial^4 w}{\partial x^2 \partial y^2} + \frac{\partial^4 w}{\partial y^4} \right) - \left(\frac{N_x}{W} \frac{\partial^2 w}{\partial x^2} + \frac{N_y}{W} \frac{\partial^2 w}{\partial y^2} \right) = P(x, y)$$

**Axial stresses in x
and y directions**

- > When tensile stress dominates over flexural rigidity (thin, tensioned plate), the plate may be considered a membrane

$$\left(\frac{N_x}{W} \frac{\partial^2 w}{\partial x^2} + \frac{N_y}{W} \frac{\partial^2 w}{\partial y^2} \right) = -P$$

How about cantilevers?

- > **Example: residually stressed cantilever, where stress is constant throughout structure**
- > **Before release: stressed cantilever is attached to surface**
- > **After release: cantilever relieves stress by expanding or contracting to its desired length**
- > **No bending of released structure**

How about nonuniform axial stress?

- > **Nonuniform axial stress through the thickness of a beam creates a bending moment**
- > **It can arise from two sources**
 - **Intrinsic stress gradients, created during formation of the cantilever material (e.g. polysilicon)**
 - **Residual stress in thin films deposited onto the cantilever**
- > **The bending moment curls the cantilever**

Example: Cantilever with stress gradient

- > Think about it in three steps:
- Relax the average stress to zero after release
 - Compute the moment when the beam is flat
 - Compute the curvature that results from the moment

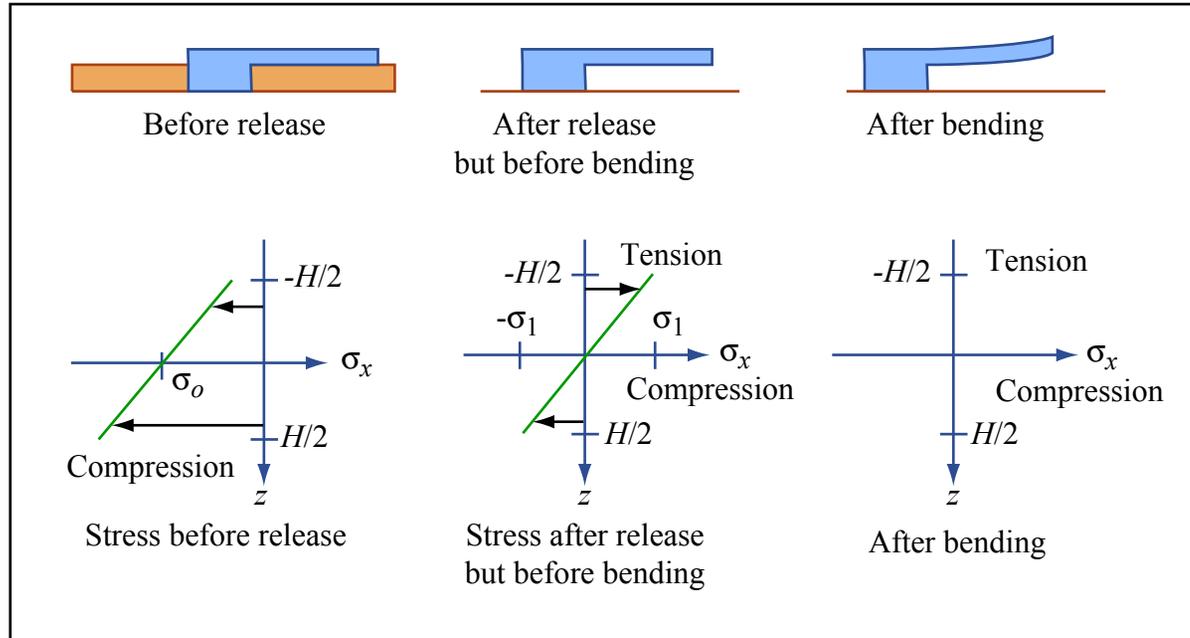


Image by MIT OpenCourseWare.

Adapted from Figure 9.13 in Senturia, Stephen D. *Microsystem Design*. Boston, MA: Kluwer Academic Publishers, 2001, p. 223. ISBN: 9780792372462.

$$M_x = \int_A z \sigma_x dA = -\frac{1}{6} WH^2 \sigma_1 \quad \text{and} \quad \rho_x = -\frac{EI}{M_x} \Rightarrow \rho_x = \frac{1}{2} \frac{EH}{\sigma_1}$$

Example: Thin Film on Cantilever

> In this case, the curling does not relieve all the stress

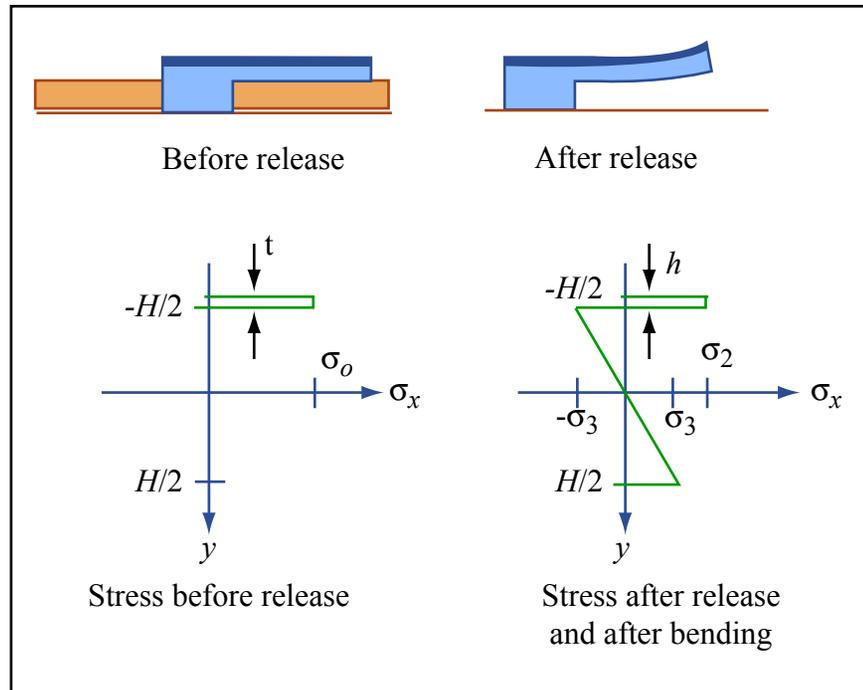
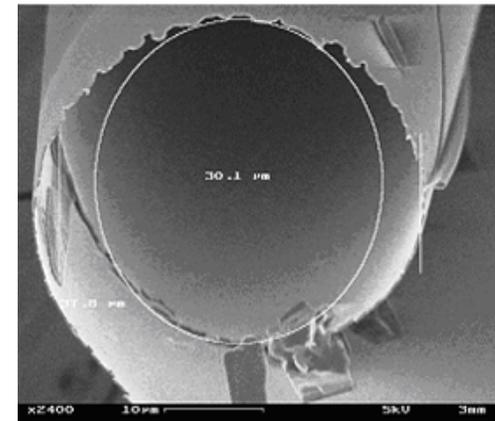


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Adapted from Figure 9.14 in Senturia, Stephen D. *Microsystem Design*.
Boston, MA: Kluwer Academic Publishers, 2001, p. 224. ISBN: 9780792372462.



> See text for math

Barbastathis group, MIT

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- > **Effects of residual stresses on structures**
- > **Energy methods**
 - **Elastic energy**
 - **Principle of virtual work: variational methods**
 - **Examples**
- > **Rayleigh-Ritz methods for resonant frequencies and extracting lumped-element masses for structures**

Elastic Energy

- > Elastic stored energy **density** is the integral of stress with respect to strain

$$\text{Elastic energy density: } \tilde{W}(x,y,z) = \int_0^{\varepsilon(x,y,z)} \sigma(\varepsilon) d\varepsilon$$

$$\text{When } \sigma(\varepsilon) = E\varepsilon: \quad \tilde{W}(x,y,z) = \frac{1}{2} E[\varepsilon(x,y,z)]^2$$

- > The total elastic stored energy is the volume integral of the elastic energy density

$$\text{Total stored elastic energy: } W = \iiint_{\text{Volume}} \tilde{W}(x,y,z) dx dy dz$$

Including Shear Strains

- > More generally, the energy density in a linear elastic medium is related to the product of stress and strain
- > A similar approach can be used for electrostatic stored energy density $(1/2)\mathbf{D}\cdot\mathbf{E}$ and magnetostatic stored energy density $(1/2)\mathbf{B}\cdot\mathbf{H}$.

$$\text{For axial strains: } \tilde{W} = \frac{1}{2} \sigma \varepsilon$$

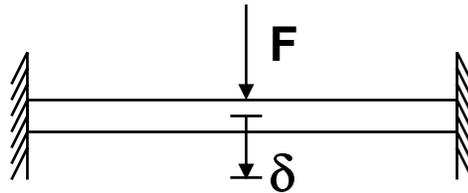
$$\text{For shear strains: } \tilde{W} = \frac{1}{2} \tau \gamma$$

This leads to a total elastic strain energy:

$$W = \frac{1}{2} \iiint_{\text{Volume}} (\sigma_x \varepsilon_x + \sigma_y \varepsilon_y + \sigma_z \varepsilon_z + \tau_{xy} \gamma_{xy} + \tau_{xz} \gamma_{xz} + \tau_{yz} \gamma_{yz}) dx dy dz$$

Concept: Principle of Virtual Work

- > The question: how to determine the deformation that results from an applied load

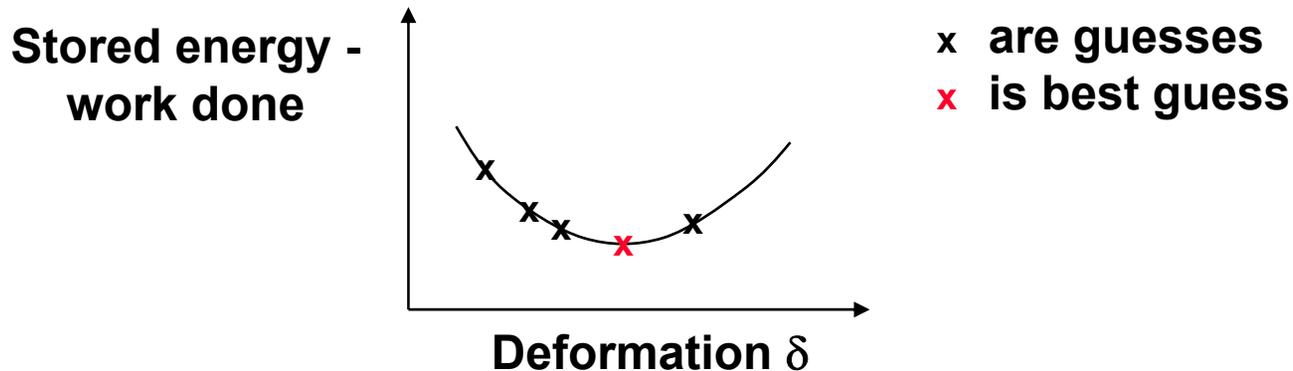


$$\delta(F) = ??$$

- > **Known: the work done on an energy-conserving system by external forces must result in an equal amount of stored potential energy**
- > Imposing this condition can provide an exact solution to many problems
 - For example, if functional dependence between quantities is known, and you just need to find what the actual values are

Concept: Principle of Virtual Work

- > Can approach this from a “guessing” point of view
 - Guess values for δ ; whichever one best equates stored energy and work done is the right answer



- > What if you don't know the functional form of your deformations/displacements – does this still work?
- > Yes! You can choose a plausible shape function for the displacement with a few adjustable parameters and iteratively “guess” the constants to best equate stored energy and work done

Principle of Virtual Work

- > **Goal: a variational method for solving energy-conserving problems (a mathematical way of approaching the “guessing”)**
- > **Define total potential U , including work and stored energy**

$$U = \text{Stored energy} - \text{Work done}$$

- > **A system in equilibrium has a total potential U that is a minimum with respect to any virtual displacement**
 - **No matter what you change, you won't get any closer to matching work and stored energy**
- > **Requirement: the virtual displacement must obey B.C.**
- > **Nomenclature for small virtual displacements**
 - **In the x direction: δu**
 - **In the y direction: δv**
 - **In the z direction: δw**

Math: Principle of Virtual Work

- > Consider all possible virtual displacements; evaluate change in strains

$$\delta\varepsilon_x = \frac{\partial}{\partial x} \delta u \quad \text{and} \quad \delta\gamma_{xy} = \left(\frac{\partial}{\partial x} \delta v + \frac{\partial}{\partial y} \delta u \right)$$

- > This implies changes in strain energy density

$$\delta\tilde{W} = \sigma_x \delta\varepsilon_x + \dots + \tau_{xy} \delta\gamma_{xy} + \dots$$

- > The principle of virtual work states that in equilibrium, for any virtual displacement that is compatible with the B.C.,

$$\begin{aligned} & \iiint_{Volume} \delta\tilde{W} dx dy dz - \iint_{Surface} (F_{s,x} \delta u + F_{s,y} \delta v + F_{s,z} \delta w) dS \\ & - \iiint_{Volume} (F_{b,x} \delta u + F_{b,y} \delta v + F_{b,z} \delta w) dx dy dz = 0 \end{aligned}$$

Differential Version

> The previous equation is equivalent to the following:

$$\delta \left[\iiint_{Volume} \tilde{W} dx dy dz - \iint_{Surface} (F_{s,x} u + F_{s,y} v + F_{s,z} w) dS - \iiint_{Volume} (F_{b,x} u + F_{b,y} v + F_{b,z} w) dx dy dz \right] = 0$$

This can be restated in the following form :

$$\delta U = 0$$

where

$$U = \left[\iiint_{Volume} \tilde{W} dx dy dz - \iint_{Surface} (F_{s,x} u + F_{s,y} v + F_{s,z} w) dS - \iiint_{Volume} (F_{b,x} u + F_{b,y} v + F_{b,z} w) dx dy dz \right]$$

Variational methods

- > **Select a trial solution with parameters that can be varied**
 - $\hat{u}(x, y, z; c_1, c_2, \dots, c_n)$ = trial displacement in x
 - $\hat{v}(x, y, z; c_1, c_2, \dots, c_n)$ = trial displacement in y
 - $\hat{w}(x, y, z; c_1, c_2, \dots, c_n)$ = trial displacement in z
- > **Formulate the total potential U of the system as functions of these parameters**
- > **Find the potential minimum with respect to the values of the parameters**

$$\frac{\partial U}{\partial c_1} = 0, \quad \frac{\partial U}{\partial c_2} = 0, \dots, \quad \frac{\partial U}{\partial c_n} = 0$$

- > **The result is the best solution possible with the assumed trial function**

Why Bother?

- > **Nonlinear partial differential equations are basically very nasty.**
- > **Approximate analytical solutions can *always* be found with variational methods**
- > **The analytical solutions have the correct dependence on geometry and material properties, hence, serve as the basis for good macro-models**
- > **Accurate numerical answers may require finite-element modeling**

Analytic vs. Numerical

- > **Analytic variational methods and numerical finite-element methods both depend on the Principal of Virtual Work**
- > **Both methods minimize total potential energy**
- > **FEM methods use **local** trial functions (one per element). Variational parameters are the nodal displacements**
- > **Analytic methods use **global** trial functions**

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Example: fixed-fixed beam, small deflections

- > **Doubly-fixed beam with a point load at some position along the beam, in the small deflection limit**
- > **Our present choice: use a fourth degree polynomial trial solution**

$$\hat{w}(x) = c_0 + c_1x + c_2x^2 + c_3x^3 + c_4x^4$$

Boundary conditions: $w = 0$ and $w' = 0$ at $x = 0, L$

- > **Apply boundary conditions:**
 - **$c_0 = c_1 = 0$ from BC at $x = 0$**
 - **BC at $x = L$ eliminate two more constants**
 - **Result is a shape function with one undetermined amplitude parameter**

$$\hat{w}(x) = c_4(L^2x^2 - 2Lx^3 + x^4)$$

Example: fixed-fixed beam, small deflections

- > Formulate total potential energy and find the minimum
- > Calculate strain energy from bending

width of beam \rightarrow $\varepsilon = -\frac{z}{\rho} = -z \frac{d^2 \hat{w}}{dx^2} = -z c_4 (2L^2 - 12Lx + 12x^2)$

total strain energy \rightarrow $W = \frac{EW}{2} \int_0^L \int_{-H/2}^{H/2} \varepsilon^2 dx dz = \frac{1}{30} EWH^3 L^5 c_4^2$

- > Calculate work done by external force applied at x_0

$$Work = F \hat{w}(x_0) = F c_4 (L^2 x_0^2 - 2Lx_0^3 + x_0^4)$$

- > This yields total potential energy

$$U = \frac{1}{30} EWH^3 L^5 c_4^2 - (L^2 x_0^2 - 2Lx_0^3 + x_0^4) F c_4$$

Example: fixed-fixed beam, small deflections

- > Minimize total potential energy with respect to c_4 , determine c_4 , and plug in to find variational solution for deflection $w(x)$

$$\frac{\partial U}{\partial c_4} = 0$$

$$c_4 = 15 \frac{L^2 x_0^2 - 2Lx_0^3 + x_0^4}{EWH^3 L^5} F$$

$$w = 15 \frac{(L^2 x_0^2 - 2Lx_0^3 + x_0^4)(L^2 x^2 - 2Lx^3 + x^4)}{EWH^3 L^5} F$$

- > Compare stiffness for the case of a center-applied load

$$w\left(\frac{L}{2}\right) = \frac{15}{256} \frac{L^3}{EWH^3} F$$

Recall solution of beam equation

$$k = \frac{256}{15} \frac{EWH^3}{L^3} \approx 17 \frac{EWH^3}{L^3}$$

$$k = 16 \frac{EWH^3}{L^3}$$

Properties of the Variational Solution

- > Does it solve the beam equation? **NO**
- > Is the point of maximum deflection near where the load is applied? **NOT IN GENERAL**
- > How can we determine how accurate the solution is? **TRY A BETTER FUNCTION**
- > Was this a good trial function? **NO**

A Better Trial Function

- > Fifth-order polynomial allows both the amplitude and shape of the deformation to be varied

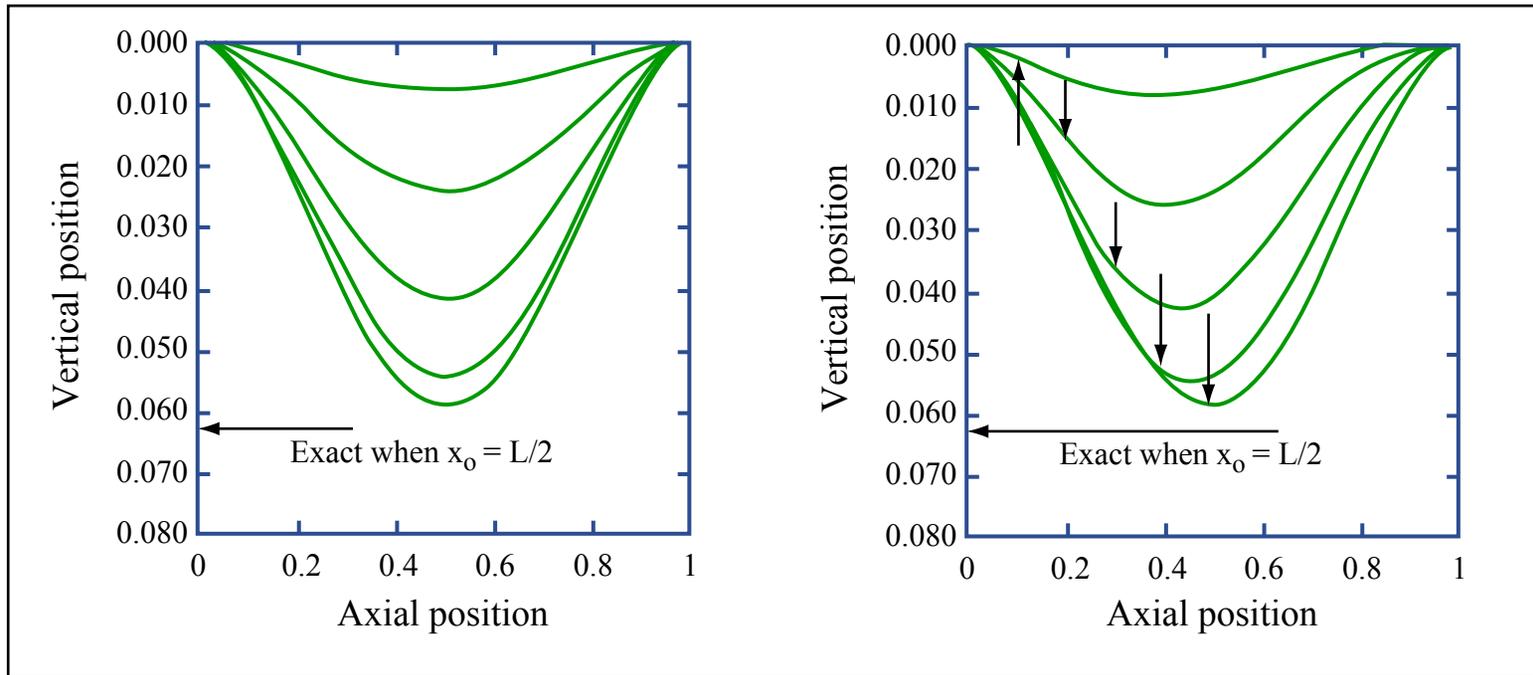


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Adapted from Figure 10.1 in Senturia, Stephen D. *Microsystem Design*. Boston, MA: Kluwer Academic Publishers, 2001, p. 248. ISBN: 9780792372462. The artist's representation of the fourth and fifth degree polynomials is approximate.

Fourth degree polynomial

Fifth degree polynomial

What about large deflections?

- > For small deflections, pure bending is a good approximation
 - The geometrically constructed neutral axis really does have about zero strain
- > For large deflections, the beam gets longer
 - Tensile side gets even more tensile
 - Compressed side gets less compressed
 - Neutral axis becomes tensile
- > We can treat this as a superposition of two events
 - First, the beam bends in pure bending, which draws the end of the beam away from the second support
 - Then, the beam is stretched to reconnect with the second support
 - Quantify the stretching by the strain at the originally neutral axis

Analysis of “Large” Deflections

- > When deflections are “large,” on the order of the beam thickness, stretching becomes important

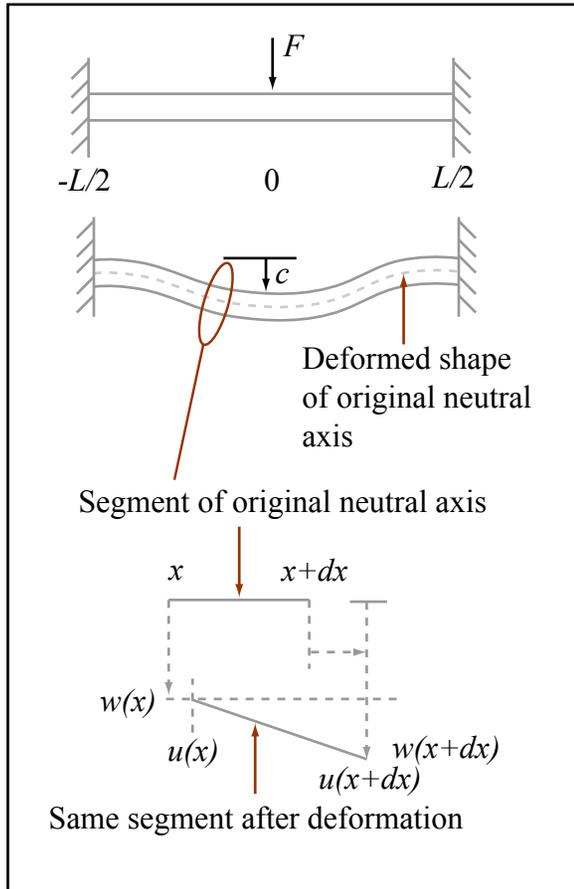


Image by MIT OpenCourseWare.

Adapted from Figure 10.2 in Senturia, Stephen D. *Microsystem Design*. Boston, MA: Kluwer Academic Publishers, 2001, p. 250. ISBN: 9780792372462.

As a result of stretching, the arc length increases

$$ds = \sqrt{[dx + u(x+dx) - u(x)]^2 + [w(x+dx) - w(x)]^2}$$

Using the result that $\sqrt{1 + \delta} = 1 + \frac{\delta}{2}$

$$ds = dx \left[1 + \frac{du}{dx} + \frac{1}{2} \left(\frac{dw}{dx} \right)^2 \right]$$

The axial strain is given by

$$\epsilon_x = \frac{ds - dx}{dx} = \frac{du}{dx} + \frac{1}{2} \left(\frac{dw}{dx} \right)^2$$

The change in length is

$$\delta L = \int_{-L/2}^{L/2} \frac{ds - dx}{dx} dx = \int_{-L/2}^{L/2} \epsilon_x dx$$

Example: Center-Loaded Beam

> Potential energy has three terms:

- Bending strain energy
- Stretching strain energy
- External work

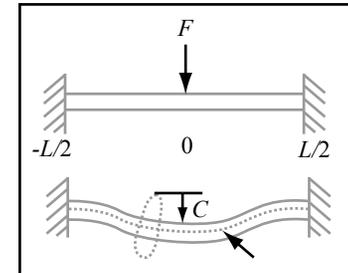


Image by MIT OpenCourseWare.

Adapted from Figure 10.2 in Senturia, Stephen D. *Microsystem Design*. Boston, MA: Kluwer Academic Publishers, 2001, p. 250. ISBN: 9780792372462.

> Bending and external work already calculated for one trial function

> Pick another trial function (same weakness as last attempt, but easy to use) and include large deflections

$$\hat{w} = \frac{c}{2} \left(1 + \cos \frac{2\pi x}{L} \right)$$

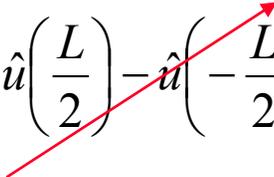
Why not a \hat{u} ?

Example: Center-Loaded Beam

- > First, calculate the strain due to stretching (aggregate axial strain)

$$\varepsilon_a = \frac{\delta L}{L} = \frac{1}{L} \int_{-L/2}^{L/2} \varepsilon_x dx$$

$$\varepsilon_a = \frac{1}{L} \int_{-L/2}^{L/2} \left[\frac{d\hat{u}}{dx} + \frac{1}{2} \left(\frac{d\hat{w}}{dx} \right)^2 \right] dx$$

$$\varepsilon_a = \frac{1}{L} \left[\hat{u} \left(\frac{L}{2} \right) - \hat{u} \left(-\frac{L}{2} \right) \right] + \frac{1}{L} \int_{-L/2}^{L/2} \left[\frac{1}{2} \left(\frac{d\hat{w}}{dx} \right)^2 \right] dx$$


- > Total strain = bending strain + aggregate axial strain

$$\varepsilon_T = \varepsilon_{bending} + \varepsilon_{stretching}$$

$$\varepsilon_T = -z \frac{d^2 \hat{w}}{dx^2} + \varepsilon_a$$

Example: Center-Loaded Beam

- > Calculate total stored elastic energy from total strain

$$W = \frac{EW}{2} \int_{-H/2}^{H/2} \int_{-L/2}^{L/2} \varepsilon_T^2 dx dz = \frac{EWH\pi^4(8H^2 + 3c^2)c^2}{96L^3}$$

- > Finally, potential energy...

$$U = W - Fc = \frac{EWH\pi^4(8H^2 + 3c^2)c^2}{96L^3} - Fc$$

- > ...which we minimize with respect to c

$$\frac{\partial U}{\partial c} = 0 \quad \Rightarrow \quad F = \left(\frac{\pi^4}{6}\right) \left[\frac{EWH^3}{L^3}\right] c + \left(\frac{\pi^4}{8}\right) \left[\frac{EWH}{L^3}\right] c^3$$

- > Compare linear term with solution to beam equation: prefactor 16.2 instead of 16

Results from example

- > **Force-displacement relationship: an amplitude-stiffened Duffing spring**

$$F = \left(\frac{\pi^4}{6}\right) \left[\frac{EWH^3}{L^3}\right] c + \left(\frac{\pi^4}{8}\right) \left[\frac{EWH}{L^3}\right] c^3$$

- > **Solution shows geometry dependence; constants may or may not be correct**

$$F = C_b \left[\frac{EWH^3}{L^3}\right] c + C_s \left[\frac{EWH}{L^3}\right] c^3$$

- > **Once you've found the elastic strain energy, finding results for another load is easy**

$$Work = Fc \quad \Rightarrow \quad Work = \int_{-L/2}^{L/2} q(x) \hat{w}(x) dx$$

Example: uniform pressure load P

- > Adopt the elastic strain energy
- > Calculate the work for a uniform pressure load

$$Work = WP \int_{-L/2}^{L/2} \frac{c}{2} \left(1 + \cos \frac{2\pi x}{L} \right) dx = \frac{WLPc}{2}$$

- > Minimize U to find relationship between load and deflection

$$P = \left(\frac{\pi^4}{3} \right) \left[\frac{EH^3}{L^4} \right] c + \left(\frac{\pi^4}{4} \right) \left[\frac{EH}{L^4} \right] c^3$$

- > The geometry dependence appears!

Combining Variational and FEM Methods

- > Use the analytic variational method to find a good functional form for the result
- > Establish non-dimensional numerical parameters within the solution
- > Perform well-meshed FEM simulations over the design space
- > Fit the analytic solution to the FEM results

Residual Stress In Clamped Structures

- > Must add a new term to the elastic energy to capture the effects of the residual stress

$$\tilde{W} = \int_0^{\varepsilon_a} \sigma d\varepsilon \quad \Rightarrow \quad \tilde{W} = \int_0^{\varepsilon_a} (\sigma_0 + E\varepsilon) d\varepsilon$$

- > Now there is a residual stress term in the stored elastic energy

$$W_r = \sigma_0 W \int_{-H/2}^{H/2} dz \int_{-L/2}^{L/2} \varepsilon_a dx$$

- > For the fixed-fixed beam example, the residual stress term is:

$$W_r = \sigma_0 W L H \left(\frac{\pi^2}{4L^2} \right) c^2$$

- > This leads to a general form of the load-deflection relationship for beams, which can be extended to plates and membranes

Results for Doubly-Clamped Beam

For the case of a central point load :

$$F = \left\{ \left(\frac{\pi^2}{2} \right) \left[\frac{\sigma_0 WH}{L} \right] + \left(\frac{\pi^4}{6} \right) \left[\frac{EWH^3}{L^3} \right] \right\} c + \left(\frac{\pi^4}{8} \right) \left[\frac{EWH}{L^3} \right] c^3$$

and for the pressure loaded case :

$$P = \left\{ \pi^2 \left[\frac{\sigma_0 H}{L^2} \right] + \left(\frac{\pi^4}{3} \right) \left[\frac{EH^3}{L^4} \right] \right\} c + \left(\frac{\pi^4}{4} \right) \left[\frac{EH}{L^4} \right] c^3$$

The general form for pressure loading, useful for fitting to FEM results, is :

$$P = \left\{ C_r \left[\frac{\sigma_0 H}{L^2} \right] + C_b \left[\frac{EH^3}{L^4} \right] \right\} c + C_s \left[\frac{EH}{L^4} \right] c^3$$

Finally, we note that the stress term dominates over bending when

$$\sigma_0 \geq \frac{EH^2}{L^2}$$

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 - **Principle of virtual work: variational methods**
 - **Examples**
- > **Rayleigh-Ritz methods for resonant frequencies and extracting lumped-element masses for structures**

Estimating Resonance Frequencies

- > We have achieved part of our goal of converting structures into lumped elements
 - We can calculate elastic stiffness of almost any structure, for small and large deflections
 - But we still don't know how to find the mass term associated with structures
- > We can get the mass term from the resonance frequency and the stiffness
- > The resonance frequency comes from Rayleigh-Ritz analysis
 - In simple harmonic motion at resonance, the maximum kinetic energy equals the maximum potential energy
 - Determine kinetic energy; equate its maximum value to the maximum potential energy; find ω_0 .

Estimating Resonance Frequencies

- > Guess a time dependent trial function from $\hat{w}(x)$

$$\hat{w}(x, t) = \hat{w}(x) \cos(\omega t)$$

- > Find maximum kinetic energy from maximum velocity

$$\text{Maximum velocity: } \left(\frac{\partial \hat{w}(x, t)}{\partial t} \right)_{t=\pi/2\omega} = -\omega \hat{w}(x)$$

$$\text{Max kinetic energy, lumped: } W_{k, \max} = \frac{1}{2} m v_{\max}^2$$

$$\text{Max kinetic energy density: } \tilde{W}_{k, \max} = \frac{1}{2} \rho_m(x) \omega^2 \hat{w}(x)^2$$

$$\text{Max kinetic energy: } W_{k, \max} = \frac{\omega^2}{2} \iiint_{\text{volume beam}} \rho_m(x) \hat{w}(x)^2 dx dy dz$$

- > Calculate maximum potential energy from $\hat{w}(x)$ as before

Rayleigh-Ritz

- > The resonance frequency is obtained from the ratio of potential energy to kinetic energy, using a variational trial function
- > The result is remarkably **insensitive** to the specific trial function

$$\omega_0^2 = \frac{W_{elastic}}{\frac{1}{2} \iiint_{Volume} \rho_m(x) \hat{w}^2(x) dx}$$

Example: Tensioned Beam

> Compare two trial solutions:

- Tensioned wire – the exact solution ($1/2 \lambda$ of a cosine)
- Bent beam – a very poor solution

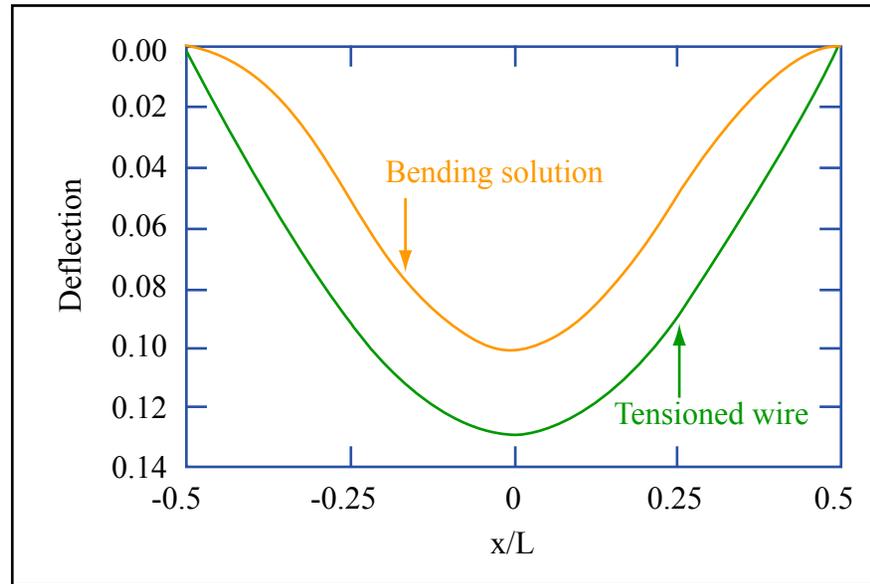


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Adapted from Figure 10.3 in Senturia, Stephen D. *Microsystem Design*. Boston, MA: Kluwer Academic Publishers, 2001, p. 263. ISBN: 9780792372462.

> Resonant frequencies differ by only 15%

> Worse trial functions yield higher stiffness, higher resonant frequencies

Extracting Lumped Masses

- > Use variational methods to calculate the stiffness
- > Use Rayleigh-Ritz with the same trial function to calculate the resonant frequency ω^2
- > Extract the mass from the relation between mass, stiffness, and resonant frequency.
 - $\omega^2 = k/m$