

1.050 Engineering Mechanics I

Lecture 29

Energy bounds in 1D systems

Examples and applications

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1.050 – Content overview

I. Dimensional analysis

1. On monsters, mice and mushrooms
2. Similarity relations: Important engineering tools

Lectures 1-3
Sept.

II. Stresses and strength

3. Stresses and equilibrium
4. Strength models (how to design structures, foundations.. against mechanical failure)

Lectures 4-15
Sept./Oct.

III. Deformation and strain

5. How strain gages work?
6. How to measure deformation in a 3D structure/material?

Lectures 16-19
Oct.

IV. Elasticity

7. Elasticity model – link stresses and deformation
8. Variational methods in elasticity

Lectures 20-31
Oct./Nov.

V. How things fail – and how to avoid it

9. Elastic instabilities
10. Plasticity (permanent deformation)
11. Fracture mechanics

Lectures 32-37
Dec.

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1.050 – Content overview

I. Dimensional analysis

II. Stresses and strength

III. Deformation and strain

IV. Elasticity

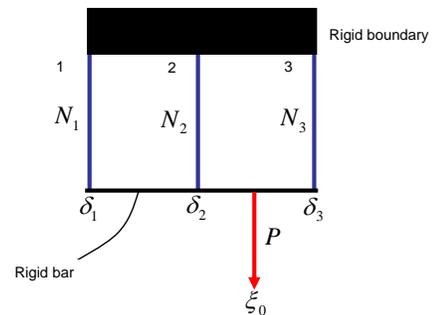
- ...
- Lecture 23: Applications and examples
- Lecture 24: Beam elasticity
- Lecture 25: Applications and examples (beam elasticity)
- Lecture 26: ... cont'd and closure
- Lecture 27: Introduction: Energy bounds in linear elasticity (1D system)
- Lecture 28: Introduction: Energy bounds in linear elasticity (1D system), cont'd
- Lecture 29: 1D examples
- Lecture 30: Generalization to 3D
- ...

V. How things fail – and how to avoid it

Lectures 32 to 37

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Example system: 1D truss structure



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Minimum potential energy approach

Conditions for kinematically admissible (K.A.): Deformation must be compatible w/ rigid bar

Consider two kinematically admissible (K.A.) displacement fields

Approximation to solution (K.A.) (1): $\delta_1' = \delta_2' = \delta_3' = \xi_0'$

Actual solution (2): $\delta_1 = \xi_0$
 $\delta_2 = \delta_1 + \frac{2}{3}(\xi_0 - \delta_1)$
 $\delta_3 = \delta_1 + \frac{4}{3}(\xi_0 - \delta_1)$

Prescribed force: P
 Unknown displacement: ξ_0

Minimum potential energy approach

$$\mathcal{E}_{\text{pot}}(\delta_i, \xi_0) = \psi(\delta_i) - P\xi_0 \leq \psi(\delta_i') - P\xi_0' = \mathcal{E}_{\text{pot}}(\delta_i', \xi_0')$$

Potential energy of actual solution is always smaller than the solution to any other displacement field

Therefore, the actual solution realizes a minimum of the potential energy:

$$\mathcal{E}_{\text{pot}}(\delta_i, \xi_i) = \min_{\delta_i, \xi_i \text{ K.A.}} \mathcal{E}_{\text{pot}}(\delta_i', \xi_i')$$

To find a solution, minimize the potential energy for a selected choice of kinematically admissible displacement fields

We have not invoked the EQ conditions!

Minimum potential energy approach

Approximation to solution (K.A.) (1): $\delta_1' = \delta_2' = \delta_3' = \xi_0'$

$$\mathcal{E}_{\text{pot}}(\xi_0') = -\frac{1}{6k} P^2$$

Actual solution (2): $\delta_1 = \xi_0$
 $\delta_2 = \delta_1 + \frac{2}{3}(\xi_0 - \delta_1)$
 $\delta_3 = \delta_1 + \frac{4}{3}(\xi_0 - \delta_1)$

$$\mathcal{E}_{\text{pot}}(\xi_0, \delta_1) = -\frac{11}{48k} P^2$$

> is larger than

Minimum complementary energy approach

Conditions for statically admissible (S.A.)

Consider two statically admissible force fields

$$\begin{cases} N_1 + N_2 + N_3 = R \\ 3N_1 + N_2 - N_3 = 0 \end{cases}$$

Approximation to solution Still S.A. (1): N_1', N_2'

Actual solution (obtained in lecture 20) (2): $N_1 = 1/12R$
 $N_2 = 1/3R$
 $N_3 = 7/12R$

Minimum complementary energy approach

$$\varepsilon_{\text{com}}(N_i, R) = \psi^*(N_i) - \xi_0^d R \leq \psi^*(N_i) - \xi_0^d R' = \varepsilon_{\text{com}}(N_i, R')$$

Complementary energy of actual solution is always smaller than the solution to any other displacement field

Therefore, the actual solution realizes a minimum of the complementary energy:

$$\varepsilon_{\text{com}}(N_i, R) = \min_{N_i, S.A.} \varepsilon_{\text{com}}(N_i, R')$$

To find a solution, minimize the complementary energy for a selected choice of statically admissible force fields

We have not invoked the kinematics of the problem!

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Minimum complementary energy approach

(1)
Approximation to solution
Still S.A.

(2)
Actual solution
(obtained in lecture 20)

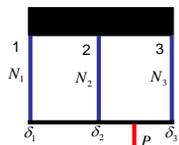
$$\varepsilon_{\text{com}}(R') = -\frac{1}{5}k(\xi_0^d)^2$$

> is larger than

$$\varepsilon_{\text{com}}(R, N_1) = -\frac{12}{11}k(\xi_0^d)^2$$

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Combine: Upper/lower bound



Prescribed force
Unknown displacement

Treat this BC with complementary energy approach

$$\varepsilon_{\text{com}}(N_1) = \psi^*(N_1) - \xi_0^d R = 0$$

$$\varepsilon_{\text{com}}(N_1) = \frac{1}{4k}(12N_1^2 - 2PN_1 + P^2)$$

Find min: $\frac{\partial \varepsilon_{\text{com}}(N_1)}{\partial N_1} = 0 \quad N_1 = \frac{1}{12}P \longrightarrow \varepsilon_{\text{com}} = \frac{11}{48k}P^2$

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Combine: Upper/lower bound

$$-\varepsilon_{\text{com}}(N_i, R') \leq \left\{ \begin{array}{l} \max_{N_i, S.A.} (-\varepsilon_{\text{com}}(N_i, R')) \\ \text{is equal to} \\ \min_{\delta_i, K.A.} \varepsilon_{\text{pot}}(\delta_i, \xi_i') \end{array} \right\} \leq \varepsilon_{\text{pot}}(\delta_i, \xi_i')$$

Lower bound Upper bound

$$\varepsilon_{\text{com}} = \frac{11}{48k}P^2 \longrightarrow -\varepsilon_{\text{com}} = -\frac{11}{48k}P^2$$

$$\varepsilon_{\text{pot}}(\xi_0, \delta_1) = -\frac{11}{48k}P^2$$

At the solution to the elasticity problem, the upper and lower bound coincide

Another example

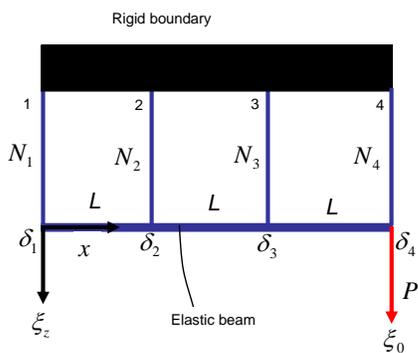
Approximate solution for coupled beam-truss problem

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Step-by-step approach

- **Step 1:** Determine K.A. displacement field (for approximation, find appropriate assumed displacement field)
- **Step 2:** Express work balance – find $\varepsilon_{\text{pot}} / \varepsilon_{\text{com}}$
- **Step 3:** Find min of $\varepsilon_{\text{pot}} / \varepsilon_{\text{com}}$
- **Step 4:** Determine displacement field, forces etc.
- **Solution is approximation to actual solution**

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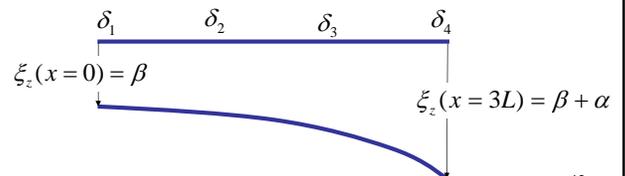
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Minimum potential energy approach

Step 1: Assume K.A. displacement field

$$\xi_z(x; \alpha, \beta) = \beta + \alpha \left(\frac{x}{3L} \right)^2$$

(approximation of the actual solution...)



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Minimum potential energy approach

Displacement of the four truss members

$$(*) \begin{cases} \delta_1 = \xi_z(x=0) = \beta \\ \delta_2 = \xi_z(x=L) = \beta + \frac{\alpha}{9} \\ \delta_3 = \xi_z(x=2L) = \beta + \frac{4}{9}\alpha \\ \delta_4 = \xi_z(x=3L) = \beta + \alpha \\ \xi_0 = \delta_4 \end{cases} \quad \xi_z(x; \alpha, \beta) = \beta + \alpha \left(\frac{x}{3L}\right)^2$$

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Minimum potential energy approach

Step 2:

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(chapter 5)

Total free energy of a beam:

$$\psi_B = \int_{x=0}^{3L} \int_{z=-h/2}^{h/2} \int_{y=-b/2}^{b/2} \frac{1}{2} E (\varepsilon_{xx}^0 + g_y^0 z)^2 dy dz dx$$

with: $\varepsilon_{xx}^0 = 0$ (no displacement in the x-direction)

$$g_y^0 = -\frac{\partial^2 \xi_z}{\partial x^2} = -\frac{2\alpha}{9L^2} \quad (\text{curvature can be calculated from the assumed displacement field})$$

$$\psi_B = \frac{E}{2} \frac{4\alpha^2}{81L^4} \int_{x=0}^{3L} \int_{z=-h/2}^{h/2} \int_{y=-b/2}^{b/2} z^2 dy dz dx \quad \psi_B(\alpha, \beta) = \underbrace{\frac{bh^3 E}{162L^3}}_{\text{"spring constant"}} \alpha^2 =: \frac{1}{2} k_B \alpha^2$$

Minimum potential energy approach

$$\psi_B(\alpha) = \frac{1}{2} k_B \alpha^2$$

Total free energy:

Sum of free energies of four trusses... $\psi_i = \frac{1}{2} k \delta_i^2$

$$\psi(\alpha, \beta) = \psi_B(\alpha) + \sum_{i=1,4} \psi_i(\alpha, \beta)$$

$$\psi(\alpha, \beta) = \frac{1}{2} k_B \alpha^2 + \frac{1}{2} k \left(\beta^2 + \left(\beta + \frac{\alpha}{9} \right)^2 + \left(\beta + \frac{4\alpha}{9} \right)^2 + (\beta + \alpha)^2 \right)$$

External work

$$W = F(\alpha + \beta)$$

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Minimum potential energy approach

$$\varepsilon_{\text{pot}}(\alpha, \beta) = \frac{1}{2} k_B \alpha^2 + \frac{1}{2} k \left(\beta^2 + \left(\beta + \frac{\alpha}{9} \right)^2 + \left(\beta + \frac{4\alpha}{9} \right)^2 + (\beta + \alpha)^2 \right) - F(\beta + \alpha)$$

$$\text{Step 3: } \min_{\alpha, \beta} (\varepsilon_{\text{pot}}(\alpha, \beta))$$

How to find minimum of this function?

Take partial derivatives, and set each to zero

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Minimum potential energy approach

$$\frac{\partial}{\partial \alpha} \left(\frac{1}{2} k_B \alpha^2 + \frac{1}{2} k \left(\beta^2 + \left(\beta + \frac{\alpha}{9} \right)^2 + \left(\beta + \frac{4\alpha}{9} \right)^2 + (\beta + \alpha)^2 \right) - F(\beta + \alpha) \right) = 0$$

$$\frac{\partial}{\partial \beta} \left(\frac{1}{2} k_B \alpha^2 + \frac{1}{2} k \left(\beta^2 + \left(\beta + \frac{\alpha}{9} \right)^2 + \left(\beta + \frac{4\alpha}{9} \right)^2 + (\beta + \alpha)^2 \right) - F(\beta + \alpha) \right) = 0$$

Results in a system of linear equations:

$$\begin{pmatrix} k_B + \frac{98}{81}k & \frac{14}{9}k \\ \frac{14}{9}k & 4k \end{pmatrix} \begin{pmatrix} \alpha \\ \beta \end{pmatrix} = \begin{pmatrix} F \\ F \end{pmatrix}$$

$$\begin{pmatrix} \alpha \\ \beta \end{pmatrix} = \begin{pmatrix} k_B + \frac{98}{81}k & \frac{14}{9}k \\ \frac{14}{9}k & 4k \end{pmatrix}^{-1} \begin{pmatrix} F \\ F \end{pmatrix}$$

Step 4: Based on solution, determine displacement field δ_i (from (*)), then forces:

$$N_i = k \delta_i$$

$$\begin{pmatrix} \alpha \\ \beta \end{pmatrix} = \begin{pmatrix} \frac{99F}{2(81k_B + 49k)} \\ \frac{F(81k_B - 28k)}{4k(81k_B + 49k)} \end{pmatrix} \quad 21$$