

16.21 Techniques of Structural Analysis and Design  
 Spring 2004  
 Unit #7 (continued)- Concepts of work and energy  
 Strain energy and potential energy of a beam

Raúl Radovitzky

March 2, 2005

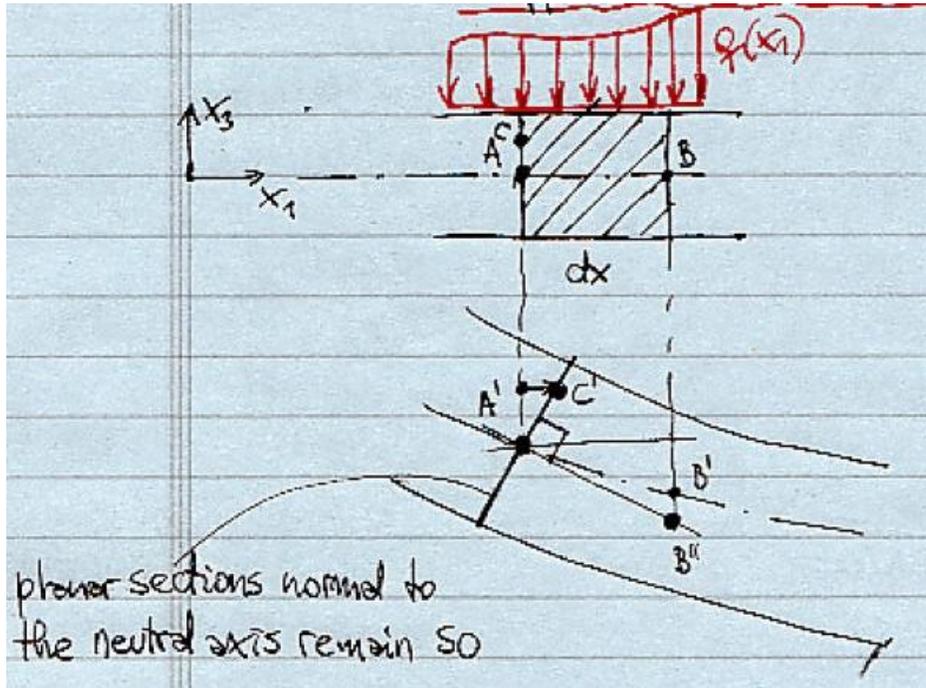


Figure 1: Kinematic assumptions for a beam

**Kinematic assumptions for a beam:** From the figure:  $\bar{A}A' = u_3(x_1)$ .  
 Assume small deflections:  $B' \sim B''$ ,  $B\bar{B}'' = u_3 + du_3$ .  $\bar{C}C' = u_3(x) +$

$u_1(x_1, x_3)$ . Assume *planar sections normal to the neutral axis remain planar after deformation*. Then:

$$u_3 = u_3(x_1) \quad (1)$$

$$u_1(x_1, x_3) = -x_3 \frac{du_3}{dx_1} \quad (2)$$

$$u_3(x_1) \text{ is the only primary unknown of the problem} \quad (3)$$

From these kinematic assumptions we can derive a theory for beams.

**Strains:**

$$\boxed{\epsilon_{11} = \frac{du_1}{dx_1} = -x_3 \frac{d^2u_3}{dx_1^2}} \quad (4)$$

$$\epsilon_{22} = \epsilon_{33} = -\nu\epsilon_{11}, \text{ plane stress} \quad (5)$$

$$\epsilon_{13} = \frac{1}{2} \left( \frac{du_1}{dx_3} + \frac{du_3}{dx_1} \right) = \frac{1}{2} \left( -\frac{du_3}{dx_1} + \frac{du_3}{dx_1} \right) = 0 \quad (6)$$

**Constitutive:**

$$\sigma_{11} = E\epsilon_{11} = -Ex_3 \frac{d^2u_3}{dx_1^2} \quad (7)$$

**Equilibrium:** Apply equilibrium (in the undeformed configuration) to *integral quantities* (moment  $M$  and shear force  $V$ ). Definitions of integral quantities as forces “equivalent” to the internal stresses:

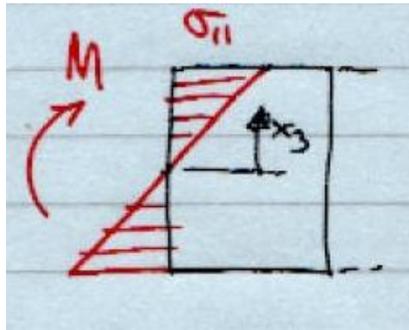
$$V(x_1) + \int_{A(x_1)} \sigma_{13} dA = 0 \quad (8)$$

$$M(x_1) + \int_{A(x_1)} \sigma_{11} x_3 dA = 0 \quad (9)$$

replacing  $\sigma_{11}$ :

$$M(x_1) = \int_{A(x_1)} \left( -E \frac{d^2u_3}{dx_1^2} x_3^2 \right) dA = E \frac{d^2u_3}{dx_1^2} \underbrace{\int_{A(x_1)} x_3^2 dA}_{I} \quad (10)$$

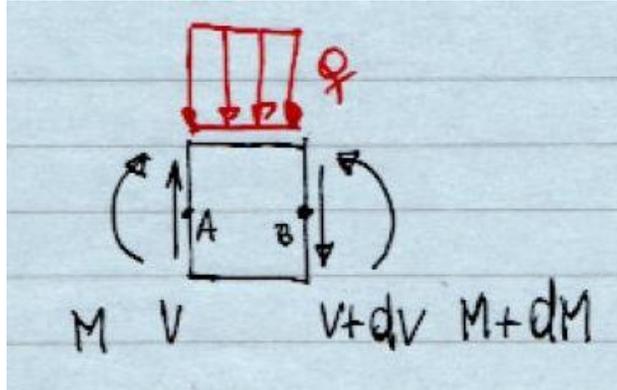
$$\boxed{M(x_1) = EI(x_1) \frac{d^2u_3}{dx_1^2}} \quad (11)$$



Also note:

$$\sigma_{11} = -\frac{Mx_3}{I} \quad (12)$$

With these definitions we can apply equilibrium as shown in the figure:



$$\sum F_{x_3} = 0 : V - qdx_1 - V - dV = 0 \rightarrow \frac{dV}{dx_1} = -q \quad (13)$$

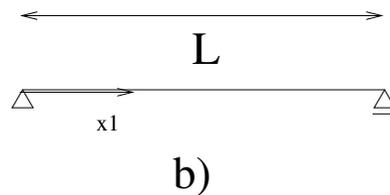
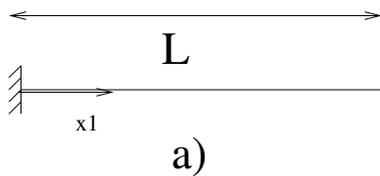
$$\sum M^B = 0 : -M + M + dM - Vdx_1 + q\frac{dx_1^2}{2} = 0 \rightarrow \frac{dM}{dx_1} = V \quad (14)$$

$$\frac{d}{dx_1} \left( \frac{dM}{dx_1} \right) = -q \rightarrow \frac{d^2M}{dx_1^2} = -q \quad (15)$$

Replacing equation (11) in the last expression:

$$\frac{d^2}{dx_1^2} \left( EI \frac{d^2u_3}{dx_1^2} \right) + q(x_1) = 0 \quad (16)$$

Fourth order differential equation governing the deflections of beams. Needs 4 boundary conditions. Examples:



- case a  $u_3(0) = 0, u_3'(0) = 0, u_3''(L) = 0, u_3'''(L) = 0.$
- case b  $u_3(0) = 0, u_3''(0) = 0, u_3(L) = 0, u_3''(L) = 0.$

**Strain energy of a beam** Start from the general definition of strain energy density:

$$U_0 = \int_0^{\epsilon_{ij}} \sigma_{ij} d\epsilon_{ij} \quad (17)$$

for a linear elastic material we concluded:

$$U_0 = \frac{1}{2} \sigma_{ij} \epsilon_{ij} \quad (18)$$

Classical beam theory:  $\sigma_{11} \neq 0$ , all other stress components are zero.

$$U_0 = \frac{1}{2} \sigma_{11} \epsilon_{11} = \frac{1}{2} E \epsilon_{11}^2 \quad (19)$$

$$\epsilon_{11} = -x_3 \frac{d^2 u_3}{dx_1^2} \rightarrow U_0 = \frac{1}{2} E x_3^2 \left( \frac{d^2 u_3}{dx_1^2} \right)^2 \quad (20)$$

$$(21)$$

$$U = \int_V U_0 dV = \int_V \frac{1}{2} E x_3^2 \left( \frac{d^2 u_3}{dx_1^2} \right)^2 dV \quad (22)$$

$$= \frac{1}{2} \int_0^L E \left( \frac{d^2 u_3}{dx_1^2} \right)^2 \int_{A(x)} x_3^2 dA dx_1 \quad (23)$$

$$\boxed{U = \frac{1}{2} \int_0^L EI(x) \left( \frac{d^2 u_3}{dx_1^2} \right)^2 dx_1} \quad (24)$$

$$\text{also note:} \quad (25)$$

$$\boxed{U = \frac{1}{2} \int_0^L M(x_1) \frac{d^2 u_3}{dx_1^2} dx_1} \quad (26)$$

### Complementary strain energy of a beam

Complementary strain energy density:

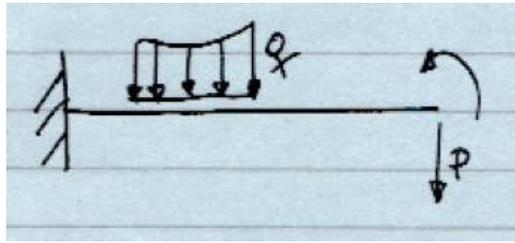
$$U_0^* = \int_0^{\epsilon_{11}} \epsilon_{11} d\sigma_{11} = \frac{1}{2} \frac{\sigma_{11}^2}{E} = \frac{1}{2E} \left( \frac{-Mx_3}{I} \right)^2 \quad (27)$$

The complementary strain energy is then

$$U_c = \int_V U_0^* dV = \frac{1}{2} \int_0^L \frac{M^2}{EI^2} \int_{A(x_1)} x_3^2 dA dx_1 \quad (28)$$

$$\boxed{U_c = \frac{1}{2} \int_0^L \frac{M^2}{EI} dx_1} \quad (29)$$

**Potential of the external forces:**



$$V = - \int_S t_i u_i dS - \int_V f_i u_i dV \quad (30)$$

$$V = - \int_0^L q(x_1) u_3(x_1) dx_1 - P u_3(L) - M u'_3(L) \quad (31)$$

The total potential energy of the beam is:

$$\Pi(u_3) = \int_0^L \left[ \frac{1}{2} EI \left( \frac{d^2 u_3}{dx^2} \right)^2 dx_1 + q u_3 \right] + P u_3(L) + M u'_3(L) \quad (32)$$

This is the expression we gave the first day of class!